

# THP CANCER CARE EQUIPMENT

ENTUITIVE

DRAWING LIST	
DRAWING No.	DRAWING TITLE
S000	COVER SHEET
S001	GENERAL NOTES
S010	TYPICAL DETAILS
S011	TYPICAL DETAILS
S012	TYPICAL DETAILS
S200	OVERALL FRAMING PLAN - LEVEL 1
S201	ENLARGED FRAMING PLAN - LEVEL 1 NORTH
S202	ENLARGED FRAMING PLAN - LEVEL 1 EAST
S400	FOUNDATION SECTIONS & DETAILS
Grand Total: 9	

3D VIEWS ARE PROVIDED TO AID CLARITY  
AND MAY NOT BE COMPLETE. REFER TO  
PLANS, SECTIONS AND SPECIFICATIONS

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PROVINCE OF ONTARIO

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9	ISSUED FOR TENDER	2025/12/16
8	ISSUED FOR PERMIT	2025/11/28
7	MOH 2.3 RE-SUBMISSION	2025/06/13
6	MOH 2.3 SUBMISSION	2024/10/11
5	ISSUED FOR 100% CONSTRUCTION DOCUMENTS	2024/09/13
4	MOH 2.3 COSTING SUBMISSION	2024/06/17
3	ISSUED FOR PROGRESS	2024/06/04
2	ISSUED FOR PROGRESS	2024/05/03
1	MOH SUBMISSION	2023/10/18
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PROJECT: THP CANCER CARE EQUIPMENT 2200 Eglinton Ave W, Mississauga, ON L5M 2N1		
TITLE: COVER SHEET		
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A. GENERAL

- WHERE DOCUMENTS ARE REFERENCED IN THE GENERAL AND DESIGN NOTES, THEY SHALL BE THE LATEST EDITIONS OR REVISION, UNLESS NOTED OTHERWISE.
- READ STRUCTURAL DOCUMENTS IN CONJUNCTION WITH EXISTING STRUCTURAL DRAWINGS, ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND OTHER CONTRACT DOCUMENTS.
  - A COPY OF THE EXISTING STRUCTURAL DRAWINGS BY HALSALL ASSOCIATES LIMITED DATED 2003/01/27 IS AVAILABLE FROM THE CONSULTANT. THIS INFORMATION IS GIVEN SOLELY AS A GUIDE. NO RESPONSIBILITY IS ACCEPTED BY THE OWNER OR THE CONSULTANT FOR ITS CORRECTNESS, NOR SHALL ITS ACCURACY OR ANY OMISSIONS AFFECT THE PROVISION OF THIS CONTRACT.
- BEFORE PROCEEDING WITH THE WORK, CHECK ALL DIMENSIONS SHOWN ON THE STRUCTURAL DOCUMENTS WITH SITE CONDITIONS AND THOSE SHOWN ON ARCHITECTURAL, MECHANICAL AND ELECTRICAL DOCUMENTS AND REPORT DISCREPANCIES TO THE CONSULTANT.
- PROVIDE LABOUR, MATERIALS, PLANT AND EQUIPMENT TO COMPLETE ALL STRUCTURAL WORK INDICATED ON THE CONTRACT DOCUMENTS.
- CARRY OUT CONSTRUCTION OPERATIONS, INCLUDING THE INSTALLATION OF TEMPORARY GUYING AND SHORING REQUIRED, ENSURING THAT THE EXISTING STRUCTURE OR MEMBERS ALREADY ERECTED ARE NOT LOADED IN EXCESS OF THEIR SAFE LOAD CARRYING CAPACITY.

A. REFERENCE STANDARDS/CODES AND ACTS

- CONFORM TO THE REQUIREMENTS OF THE ONTARIO BUILDING CODE 2012 (OBC), AND ANY APPLICABLE ACTS OF ANY AUTHORITY HAVING JURISDICTION, AND THE FOLLOWING:
  - CAN/CSA A23.1 CONCRETE MATERIALS AND METHODS OF CONCRETE CONSTRUCTION.
  - CAN/CSA A23.2 METHODS OF TEST FOR CONCRETE.
  - CAN/CSA A23.3 DESIGN OF CONCRETE STRUCTURES.
  - CAN/CSA-S16 DESIGN OF STEEL STRUCTURES.
  - RSIC 1996 REINFORCING STEEL INSTITUTE OF CANADA (RSIC), MANUAL OF STANDARD PRACTICE.
  - CAN/CSA G40.20/G40.21 STRUCTURAL QUALITY STEEL.
- ALL STANDARDS AND PUBLICATIONS REFERENCED BY THE STANDARDS NOTED ABOVE ARE TO APPLY.
- WHERE THERE ARE DIFFERENCES BETWEEN THE DOCUMENTS AND THE STANDARDS, CODES AND ACTS, THE MOST STRINGENT SHALL GOVERN.

C. QUALIFICATIONS

- ANY ORGANIZATION UNDERTAKING TO WELD UNDER THIS CONTRACT SHALL BE CERTIFIED BY THE CANADIAN WELDING BUREAU UNDER REQUIREMENTS OF DIVISION 1 OR DIVISION 2.1 OF W47.1.

D. SUBMITTALS

- SHOP DRAWINGS
  - SUBMIT FOR REVIEW BY THE CONSULTANT, DETAILED SHOP DRAWINGS FOR ALL TEMPORARY AND PERMANENT STRUCTURAL WORK INCLUDING, BUT NOT LIMITED TO: CONCRETE FORMWORK; REINFORCING STEEL; STRUCTURAL STEEL, INCLUDING TEMPORARY SHORING; LIGHTWEIGHT STEEL.
  - THE SCALE OF THE DRAWINGS SHALL BE SUCH THAT THE DETAILS OF THE STRUCTURAL WORK ARE CLEARLY SHOWN, AND IN NO CASE SMALLER THAN 1:50.
  - EACH DRAWING SUBMITTED FOR CONCRETE FORMWORK; STRUCTURAL STEEL INCLUDING TEMPORARY SHORING, SHALL BEAR THE SEAL AND SIGNATURE OF A QUALIFIED PROFESSIONAL ENGINEER LICENSED BY THE AUTHORITY HAVING JURISDICTION.
  - CONTRACTOR SHALL ALLOW FOR A 5 WORKING DAY TURN AROUND TIME FOR STRUCTURAL CONSULTANT TO REVIEW THE SHOP DRAWINGS.
  - CALCULATIONS: SUBMIT CALCULATIONS, BEARING THE SEAL AND SIGNATURE OF PROFESSIONAL ENGINEER LICENSED BY THE AUTHORITY HAVING JURISDICTION, FOR STRUCTURAL WORK, IF REQUESTED BY THE CONSULTANT.
- MILL TEST REPORTS: MAKE AVAILABLE TO THE CONSULTANT COPIES OF ALL MILL TEST REPORTS COVERING CHEMICAL AND PHYSICAL PROPERTIES OF MATERIALS USED.
- CONCRETE MIX DESIGNS: SUBMIT ALL CONCRETE MIX DESIGNS FOR REVIEW. DESCRIBE IN DETAIL ON THE MIX DESIGN SUMMARY THE LOCATION(S) WHERE EACH MIX IS TO BE PLACED IN THE STRUCTURE.
- AS-BUILT DRAWINGS: MARK ON A COMPLETE SET OF REPRODUCIBLE AS-BUILT DRAWINGS ANY CHANGES, ADDITIONS, OR DELETIONS THAT OCCUR DURING CONSTRUCTION AS A RESULT OF THE CONTRACTOR'S WORK, CHANGE OF ORDERS OR FOR ANY OTHER REASON.
- SUBMIT UNDERPINNING PROCEDURES AND DETAILS.

E. MATERIALS

- PROVIDE ONLY NEW STRUCTURAL MATERIALS IN ACCORDANCE WITH THE REFERENCE STANDARDS AND THE FOLLOWING, UNLESS OTHERWISE NOTED.
  - CONCRETE: CONFORM TO THE REQUIREMENTS OF CSA-A23.1 AND THE FOLLOWING:
    - EXPOSED TO FREEZE-THAW AND CHLORIDES (EXPOSURE CLASS C-1): f'c = 35 MPa.
    - EXPOSED TO FREEZE-THAW (EXPOSURE CLASS F-1): f'c = 30 MPa.
    - NOT EXPOSED: f'c = 25 MPa.
  - REINFORCEMENT: CONFORM TO CSA G30 SERIES, Fy = 400 MPa FOR ALL REINFORCEMENT. ALL REINFORCEMENT IS TO BE BLACK EXCEPT WHERE THE SUFFIX C IS USED TO DESIGNATE EPOXY COATED REINFORCEMENT.
  - WELDED WIRE FABRIC: CONFORM TO CSA G30 SERIES, GRADE 386, IN FLAT SHEETS.
  - STRUCTURAL STEEL:
    - STRUCTURAL WIDE FLANGE AND WELDED WIDE FLANGE SHAPES (W, WWF) TO CONFORM TO CAN/CSA-G40.20/G40.21 GRADE 350W.
    - ANGLES AND CHANNELS (L, C) TO CONFORM TO CAN/CSA-G40.20/G40.21 GRADE 300W.
    - HOLLOW STRUCTURAL SECTIONS (HSS) TO CONFORM TO ASTM A500 GRADE C.
  - HOT DIP GALVANIZING: CONFORM TO CSA G164, MINIMUM ZINC COATING OF 600 g/m<sup>2</sup>.
  - STRUCTURAL BOLTS SHALL CONFORM TO ASTM F3125 (GRADES A325, F1852, A490 AND F2280), NUTS SHALL CONFORM TO ASTM A563, WASHERS SHALL CONFORM TO ASTM F436.
  - WELDED STUD SHEAR CONNECTORS: HEADED STUDS SHALL BE MANUFACTURED BY NELSON (OR APPROVED EQUIVALENT) AND SHALL BE MADE FROM ASTM A-108 COLD ROLLED, DEFORMED WIRE MEETING THE MECHANICAL PROPERTIES OF ASTM A-496 AND SHALL BE WELDED PER THE MANUFACTURER'S RECOMMENDATIONS. STUDS SHALL BE 19 mm IN DIAMETER AND SHALL HAVE A LENGTH (AFTER WELDING) OF 75 mm WHEN 38 mm DECK IS SPECIFIED AND 115 mm WHEN 76 mm DECK IS SPECIFIED.
  - ANCHOR BOLTS: GRADE A307 OR 300W THREADED ROD CONFORMING TO CSA G40.21-M.
  - CONCRETE ANCHORS: HEADED STUDS MADE FROM ASTM A-108 COLD ROLLED DEFORMED WIRE MEETING ASTM A-496.
  - STEEL DECK: CONFORM TO ASTM A653M GRADE A OR B, MINIMUM STEEL CORE THICKNESS OF 0.76 mm. ACTUAL STEEL CORE THICKNESS IS TO BE DETERMINED BY THE SUPPLIER'S ENGINEER AND SHALL SATISFY ALL REQUIRED DESIGN CRITERIA. PROTECTIVE COATING - WIPE COATED STEEL DESIGNATION ZF075.
  - RIGID INSULATION (FOR USE WITH FOUNDATIONS AND THE LIKE): EXTRUDED POLYSTYRENE WITH A MINIMUM COMPRESSIVE STRENGTH OF 0.24 MPa UNLESS NOTED OTHERWISE.

F. EXECUTION

- FOUNDATIONS
  - A COPY OF THE SOIL INVESTIGATION REPORT BY PETO MACCALLUMN LIMITED DATED 2001/08/24 IS AVAILABLE FROM THE CONSULTANT. READ THIS REPORT, VISIT THE SITE AND THOROUGHLY FAMILIARIZE YOURSELF WITH ALL SURFACE AND SUBSURFACE CONDITIONS. THIS INFORMATION IS GIVEN SOLELY AS A GUIDE. NO RESPONSIBILITY IS ACCEPTED BY THE OWNER OR THE CONSULTANT FOR ITS CORRECTNESS, NOR SHALL ITS ACCURACY OR ANY OMISSIONS AFFECT THE PROVISION OF THIS CONTRACT.
  - FOUND ALL FOOTINGS (AND UNDERPINNING) ON SOIL CAPABLE OF SUSTAINING AN UNFACTORED BEARING STRESS OF 400 kN/m<sup>2</sup>.
  - FOUND ALL FOOTINGS WHICH WILL BE EXPOSED TO FROST ACTION IN THE COMPLETED BUILDING A MINIMUM OF 1200 mm BELOW FINISHED GRADE.
  - DO NOT EXCEED A RISE OF 7 IN A RUN OF 10 IN THE LINE OF SLOPE BETWEEN ADJACENT FOOTING EXCAVATIONS OR ALONG STEPPED FOOTINGS. FOR STEPPED FOOTINGS, USE STEPS NOT EXCEEDING 600 mm IN HEIGHT AND 1200 mm (MIN.) IN LENGTH.
  - SOIL BEARING CAPACITY SPECIFIED MUST BE VERIFIED IN WRITING BY THE SOIL ENGINEER PRIOR TO THE PLACING OF FOOTINGS AND ANY NON-CONFORMANCE WITH THE SPECIFIED MINIMUM CAPACITIES MUST BE IMMEDIATELY REPORTED TO THE STRUCTURAL ENGINEERS.
  - BELOW SLABS ON GRADE BACKFILL USING NATIVE MATERIALS OR ENGINEERED FILL APPROVED BY THE GEOTECHNICAL CONSULTANT AND COMPACT IN MAX 150 mm LIFTS TO 98% SPMDD.
  - PROVIDE TEMPORARY FROST PROTECTION, DURING CONSTRUCTION, FOR ALL FOUNDATIONS WHICH ARE NOT FOUNDED A MINIMUM OF 1200 mm BELOW GRADE.
  - FOUND NEW FOOTINGS WHICH ARE LOCATED ADJACENT TO EXISTING FOOTINGS, AT THE SAME ELEVATION AS THE EXISTING FOOTINGS, UNLESS NOTED OTHERWISE.
  - INSULATION IS SHOWN WHERE REQUIRED FOR PROTECTION OF THE FOUNDATIONS FROM DAMAGE DUE TO FROST ACTION ONLY. REFER TO ARCHITECTURAL DRAWINGS FOR FOUNDATION INSULATION NOT SHOWN ON THE STRUCTURAL DRAWINGS.
  - DO NOT COMPACT CLOSER THAN 1800 mm FROM WALLS WITH HEAVY EQUIPMENT. USE LIGHT HAND CONTROLLED EQUIPMENT WITHIN 1800 mm FROM WALLS.
- SLAB-ON-GRADE
  - THE EXISTING SLAB-ON-GRADE HAS BEEN ASSUMED TO SUSTAIN A MINIMUM SLS BEARING PRESSURE OF 25 kPa WITHOUT SETTLEMENT.

3 CONCRETE

- THE CONTRACTOR SHALL ENSURE THAT REINFORCING STEEL IS ADEQUATELY BRACED AGAINST MOVEMENT DURING CONCRETE PLACING.
- FABRICATE REINFORCEMENT IN ACCORDANCE WITH CAN/CSA A23.1 AND THE RSIC MANUAL OF STANDARD PRACTICE.
- PERFORM FORMING OPERATION AND PLACE HARDWARE SO THAT FINISHED CONCRETE WILL BE WITHIN THE TOLERANCES SET OUT IN CAN/CSA A23.1.
- FOLLOW MANUFACTURER'S INSTRUCTIONS REGARDING INSTALLATION PROCEDURES AND MINIMUM EMBEDMENT OF ANCHORS.
- GROUT BENEATH PLATES BEARING ON CONCRETE WITH AN APPROVED NON-SHRINK FLOWABLE GROUT. CONFORM TO THE MANUFACTURER'S DIRECTIONS FOR MIXING AND PLACING GROUT. COMPLETELY FILL VOIDS BENEATH STEEL BASES ON CONCRETE WITH AN APPROVED NON-SHRINK 35 MPa GROUT.
- REINFORCEMENT IDENTIFIED AS 'CONTINUOUS' SHALL TERMINATE WITH STANDARD END HOOKS AND SHALL BE LAPPED WITH CLASS 'B' TENSION LAP SPICES.
- REINFORCEMENT LENGTHS NOTED IN TYPICAL DETAILS ARE MINIMUM LENGTHS UNLESS NOTED OTHERWISE.
- OPENINGS, SLEEVES, EMBEDDED DUCTS:
  - NO OPENINGS SHALL BE MADE IN EXISTING PRECAST SLAB OR FOUNDATION WALLS UNLESS REVIEWED AND APPROVED BY THE CONSULTANT
- LAP SPICES FOR WELDED WIRE FABRIC (WWF) SHALL BE:
  - 152X152 WWF 500 mm
  - 102X102 WWF 350 mm
  - 51X51 WWF 250 mm
- CONCRETE COVER:
  - COVER SHALL BE MEASURED FROM THE DEEPEST POINT TEXTURED CONCRETE SURFACE (OR REGLET/REVEAL) TO THE NEAREST DEFORMATION OF REINFORCEMENT. REINFORCEMENT INCLUDES TIES / STIRRUPS AND MAIN REINFORCEMENT.
  - ALL CONCRETE CAST AGAINST EARTH IS TO HAVE 75 mm COVER, UNLESS NOTED OTHERWISE.
  - ALL CONCRETE EXPOSED TO EARTH, INCLUDING CONCRETE CAST AGAINST FORMS AND SUBSEQUENTLY EXPOSED TO EARTH, IS TO HAVE 50 mm COVER, UNLESS NOTED OTHERWISE.
- WHERE REINFORCEMENT IS NOT SPECIFICALLY IDENTIFIED ON THE DRAWINGS, PROVIDE 152 X 152 MW18.7 X MW18.7 WELDED WIRE FABRIC AT IN SLABS ON GRADE, OR WALKS AND 51 X 51 MW5.6 X MW5.6 TOPPING5 60 mm IN THICKNESS OR GREATER.
- PLACING CONCRETE
  - CONFORM TO REQUIREMENTS OF CSA A23.1. AND THE FOLLOWING:
    - IMMEDIATELY BEFORE PLACING CONCRETE, CLEAN FORMS AND REINFORCEMENT OF FOREIGN MATTER.
    - DO NOT USE CONCRETE MIXED MORE THAN TWO HOURS AFTER INTRODUCTION OF MIXING WATER.
    - DURING HOT WEATHER CONDITIONS, DO NOT USE CONCRETE MIXED MORE THAN ONE HOUR AFTER INTRODUCTION OF MIXING WATER.
    - ALLOW 24 HOURS MINIMUM AFTER PLACING CONCRETE IN COLUMNS, PIERS, OR WALLS BEFORE PLACING CONCRETE IN BEAMS OR SLABS SUPPORTED THEREON.
  - PLACE CONCRETE ON AND STEEL DECK FLOORS IN A MANNER THAT AVOIDS PILING UP OF CONCRETE. DO NOT DROP CONCRETE DIRECTLY FROM BUCKETS, BUT EMPLOY SUITABLE MEANS OF DISTRIBUTION. WET DOWN DECK DURING HOT WEATHER PRIOR TO CONCRETING.
    - REMOVE CONCRETE SPILLED ONTO FORMS AROUND HOISTING EQUIPMENT BEFORE DEPOSITING CONCRETE IN THESE AREAS.
- CURING CONCRETE
  - CURE ALL CONCRETE IN ACCORDANCE WITH CSA A23.1, THE CONCRETE SUPPLIERS REQUIREMENTS AND AS SPECIFIED HEREIN.
- PROTECTION
  - CONFORM TO THE REQUIREMENTS OF CSA A23.1. PROTECT FRESHLY DEPOSITED CONCRETE FROM FREEZING, PREMATURE DRYING AND EXTREMES OF TEMPERATURE. MAINTAIN CONCRETE WITH MINIMAL MOISTURE LOSS AT A RELATIVELY CONSTANT TEMPERATURE FOR THE PERIOD OF TIME NECESSARY FOR THE HYDRATION OF THE CEMENT AND TO ACHIEVE THE SPECIFIED STRENGTH OF THE CONCRETE.
  - PROVIDE SUFFICIENT INSULATION, AND HEAT AS NECESSARY, TO PREVENT FREEZING OF FROST SUSCEPTIBLE SOIL WHICH LIES AGAINST STRUCTURAL ELEMENTS; IN PARTICULAR PROTECT SOIL BENEATH FOOTINGS AND BEHIND FOUNDATION WALLS UNTIL THE BUILDING IS COMPLETED.
  - CRACK REPAIR: PRIOR TO COMPLETION OF THE PROJECT AND IN ANY CASE NOT SOONER THAN 28 DAYS AFTER CONCRETE HAS BEEN PLACED, EXAMINE CONCRETE FLOOR SURFACES AND REPAIR ALL MAJOR CRACKS IN THEM. ROUT CRACKS OUT WITH MECHANICAL ROUTER TO 13 mm (1/2") SQUARE APPROXIMATE CROSS SECTION. THEN CLEAN AND FILL CRACKS IN SAME MANNER AS SAW CUTS IN SLAB-ON-GRADE.
- THE USE OF SHOTCRETE TO CONSTRUCT ANY PART OF THE WORK SHALL BE AT THE SOLE DISCRETION OF THE CONSULTANT.

4 POST-INSTALLED ANCHORS

- EXCEPT WHERE INDICATED ON THE DRAWINGS, POST-INSTALLED ANCHORS SHALL CONSIST OF THE FOLLOWING ANCHOR TYPES AS PROVIDED BY HILTI (CANADA) CORPORATION.
  - ANCHORAGE TO CONCRETE:
    - ADHESIVE ANCHORS FOR CONCRETE USE:
      - HILTI HIT-HY 200 SAFE SET SYSTEM WITH HILTI HIT-Z ROD FOR FAST CURE APPLICATIONS.
      - HILTI HIT-HY 200 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT SYSTEM FOR FAST CURE APPLICATIONS.
      - HILTI HIT-RE 500-SD EPOXY ADHESIVE ANCHORING SYSTEM FOR SLOW CURE APPLICATIONS.
      - HILTI HIT-RE 500 EPOXY ADHESIVE ANCHORING SYSTEM FOR SLOW CURE APPLICATIONS.
      - STEEL ANCHOR ELEMENT SHALL BE HILTI HIS-N INTERNALLY THREADED INSERTS, HILTI HAS-E CONTINUOUSLY THREADED ROD, OR CONTINUOUSLY DEFORMED STEEL REBAR.
    - REBAR DOWELING INTO CONCRETE:
      - ADHESIVE ANCHORS FOR CRACKED AND UNCRACKED CONCRETE USE:
        - HILTI HIT-HY 200 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT SYSTEM WITH CONTINUOUSLY DEFORMED REBAR.
        - HILTI HIT-RE 500-SD EPOXY ADHESIVE ANCHORING SYSTEM WITH CONTINUOUSLY DEFORMED REBAR.
        - HILTI HIT-RE 500 EPOXY ADHESIVE ANCHORING SYSTEM WITH CONTINUOUSLY DEFORMED REBAR.
  - ANCHOR CAPACITY USED IN DESIGN HAS BEEN BASED ON THE TECHNICAL DATA PUBLISHED BY HILTI. SUBSTITUTION REQUESTS FOR ALTERNATE ANCHORS MUST BE APPROVED IN WRITING BY THE CONSULTANT PRIOR TO USE. CONTRACTOR SHALL PROVIDE CALCULATIONS DEMONSTRATING THAT THE ALTERNATIVE ANCHOR IS CAPABLE OF ACHIEVING THE PERFORMANCE VALUES OF THE SPECIFIED PRODUCT. SUBSTITUTIONS WILL BE EVALUATED FOR COMPLIANCE WITH THE RELEVANT BUILDING CODE. ADHESIVE ANCHOR EVALUATION WILL ALSO CONSIDER CREEP, IN-SERVICE TEMPERATURE, AND INSTALLATION TEMPERATURE.
  - INSTALL ANCHORS PER THE MANUFACTURER'S WRITTEN INSTRUCTIONS.
  - THE CONTRACTOR SHALL ARRANGE AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING FOR ALL OF THEIR ANCHORING PRODUCTS SPECIFIED. THE CONSULTANT MUST RECEIVE DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL ANCHORS ARE TRAINED PRIOR TO THE COMMENCEMENT OF INSTALLING ANCHORS.
  - ANCHOR CAPACITY IS DEPENDANT UPON SPACING BETWEEN ADJACENT ANCHORS AND PROXIMITY OF ANCHORS TO EDGE OF CONCRETE. INSTALL ANCHORS IN STRICT ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS.
  - EXISTING REINFORCEMENT IN THE CONCRETE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS. UNLESS NOTED ON THE DRAWINGS THAT THE BARS CAN BE CUT, THE CONTRACTOR SHALL REVIEW THE EXISTING STRUCTURAL DRAWINGS AND SHALL UNDERTAKE TO LOCATE THE POSITION OF THE EXISTING REINFORCEMENT AT THE LOCATIONS OF THE CONCRETE ANCHORS, BY HILTI FERROSCAN, HILTI PS 1000, GPR, X-RAY, CHIPPING OR OTHER MEANS.

5 ALTERATIONS AND/OR CONNECTIONS TO EXISTING STRUCTURE:

- PROPOSED SCHEDULE OF WORK TO BE COORDINATED WITH ALL SUBTRADES, THE CONSULTANT AND OWNER.
- PROPOSED SEQUENCE OF WORK TO BE SUBMITTED TO THE CONSULTANT FOR REVIEW PRIOR TO START OF WORK.
- INSPECT THE EXISTING BUILDING AND BECOME THOROUGHLY FAMILIAR WITH THE EXISTING CONDITIONS.
- PRIOR TO PROCEEDING WITH THE WORK, DETERMINE THE EXACT FOUNDING ELEVATIONS OF EXISTING FOOTINGS ADJACENT TO THE NEW WORK. REPORT THESE FINDINGS TO THE CONSULTANT.
- PRIOR TO FABRICATION OF STRUCTURAL STEEL, OPEN UP ALL AREAS WHERE CONNECTIONS ARE TO BE MADE TO EXISTING WORK AND TAKE FIELD MEASUREMENTS. MODIFY METHODS FOR CONNECTING TO SUIT SITE CONDITIONS FOUND AND TO THE APPROVAL OF THE CONSULTANT. CARRY OUT LOCAL REPAIRS TO THE EXISTING WORK AS NECESSARY AND AS DIRECTED BY THE CONSULTANT.
- SHORE EXISTING WORK AS REQUIRED UNTIL ALL NEW WORK HAS BEEN COMPLETED AND REVIEWED BY THE CONSULTANT.
- SHORE FLOORS AS REQUIRED TO SUPPORT CRANES, HOISTS AND OTHER CONSTRUCTION EQUIPMENT.
- DO NOT CUT CONCRETE REINFORCEMENT UNLESS REVIEWED AND APPROVED BY THE CONSULTANT.
- WHERE REQUIRED TO AVOID CUTTING EXISTING REINFORCEMENT, MODIFY THE LAYOUT OF NEW THROUGH BOLTS, EXPANSION ANCHORS AND OTHER ANCHORING DEVICES.
- MAKE GOOD THE EXISTING WORK.

6 CUTTING AND CORING OF EXISTING STRUCTURE

- PRIOR TO CUTTING AND CORING ANY OPENINGS IN THE EXISTING BUILDING, PROVIDE THE CONSULTANT WITH A SLEEVEING DRAWING INDICATING THE SIZE AND LOCATION OF OPENING RELATIVE TO BUILDING GRID LINES. EXISTING OPENINGS IN THE VICINITY OF THE NEW OPENING MUST ALSO BE SHOWN.
- ALL DIMENSIONS PROVIDED TO THE CONSULTANT ARE TO BE CONFIRMED WITH THE APPROPRIATE CONTRACTOR (MECHANICAL OR ELECTRICAL) PRIOR TO CUTTING/CORING.
- ANY REVISIONS TO THE DIMENSIONS BY THE CONSULTANT MUST BE REVIEWED BY THE APPROPRIATE CONTRACTOR PRIOR TO CUTTING/CORING.
- EXISTING REINFORCEMENT AND EMBEDDED SERVICES MUST BE LOCATED PRIOR TO CUTTING/CORING. THIS REINFORCEMENT IS TO BE LOCATED BY A POSITIVE MEANS, (I.E. X-RAYING, LOCAL CHIPPING OF SLAB- WHERE PERMITTED BY THE CONSULTANT-, USE OF COVER METER).
- AFTER REINFORCEMENT AND EMBEDDED SERVICES HAS BEEN LOCATED IN THESE AREAS, NOTIFY CONSULTANT WHO WILL REVIEW AND APPROVE LOCATION PRIOR TO CUTTING/CORING. MAKE ANY NECESSARY ADJUSTMENTS TO THE HOLE LOCATION AS DIRECTED BY THE CONSULTANT.
- FOR ANY OPENINGS WHICH ARE TO BE SAWCUT INTO THE EXISTING STRUCTURE, PRE-DRILL THE CORNERS USING A 100 mm DIAMETER CORE DRILL. DO NOT OVERCUT CORNERS OF OPENING.
- ALL PRICES FOR CUTTING/CORING ARE TO INCLUDE ANY COSTS ASSOCIATED WITH X-RAYING, CHIPPING, ETC.
- FOR ANY AREAS WHERE REINFORCEMENT IS CUT, THE CONTRACTOR IS TO INDICATE THE DIRECTION AND LAYER OF REINFORCEMENT ON THE AS-BUILT SLEEVEING DRAWINGS.
- FOR LARGE OPENINGS THROUGH A FLOOR AREA, ADDITIONAL REINFORCEMENT OF THE SLAB MAY BE REQUIRED. THE CONSULTANT WILL ISSUE ADDITIONAL DETAILS AS REQUIRED.

G. QUALITY CONTROL

1. GENERAL

- IMPLEMENT A SYSTEM OF QUALITY CONTROL TO ENSURE THAT THE MINIMUM STANDARDS SPECIFIED HEREIN ARE ATTAINED.
- BRING TO THE ATTENTION OF THE CONSULTANT ANY DEFECTS IN THE WORK OR DEPARTURES FROM THE CONTRACT DOCUMENTS, WHICH MAY OCCUR DURING CONSTRUCTION. THE CONSULTANT WILL DECIDE UPON CORRECTIVE ACTION AND GIVE RECOMMENDATIONS IN WRITING.
- THE CONSULTANT'S GENERAL REVIEW DURING CONSTRUCTION AND INSPECTION AND TESTING BY INDEPENDENT INSPECTION AND TESTING AGENCIES REPORTING TO THE CONSULTANT ARE BOTH UNDERTAKEN TO INFORM THE OWNER/CLIENT OF THE CONTRACTOR'S PERFORMANCE AND SHALL IN NO WAY AUGMENT THE CONTRACTOR'S QUALITY CONTROL OR RELIEVE THE CONTRACTOR OF CONTRACTUAL RESPONSIBILITY.

2. NOTIFICATION

- PRIOR TO COMMENCING SIGNIFICANT SEGMENTS OF THE WORK, GIVE THE CONSULTANT AND INDEPENDENT INSPECTION AND TESTING COMPANIES APPROPRIATE NOTIFICATION (MINIMUM 24 HOURS) SO AS TO AFFORD THEM REASONABLE OPPORTUNITY TO REVIEW THE WORK. FAILURE TO MEET THIS REQUIREMENT MAY BE CAUSE FOR THE CONSULTANT TO CLASSIFY THE WORK AS DEFECTIVE.

3. INSPECTION AND TESTING

- THE OWNER WILL APPOINT AN INDEPENDENT INSPECTION AND TESTING COMPANY TO MAKE INSPECTIONS OR PERFORM TESTS AS THE OWNER DIRECTS. THE INDEPENDENT INSPECTION AND TESTING COMPANIES SHALL BE RESPONSIBLE ONLY TO THE OWNER AND SHALL MAKE ONLY SUCH INSPECTIONS OR TESTS AS THE OWNER MAY DIRECT. AUTHORIZED INSPECTION AND TESTING SHALL BE PAID FOR BY THE OWNER.

4. DEFECTIVE MATERIALS AND WORK

- WHERE EVIDENCE EXISTS THAT DEFECTIVE WORK HAS OCCURRED OR THAT WORK HAS BEEN CARRIED OUT INCORPORATING DEFECTIVE MATERIALS, THE CONSULTANT MAY HAVE TESTS, INSPECTIONS OR SURVEYS PERFORMED, ANALYTICAL CALCULATIONS OF STRUCTURAL STRENGTH MADE, AND THE LIKE, IN ORDER TO HELP DETERMINE WHETHER THE WORK MUST BE CORRECTED OR REPLACED. TESTS, INSPECTIONS, SURVEYS, OR CALCULATIONS CARRIED OUT UNDER THESE CIRCUMSTANCES WILL BE MADE AT THE CONTRACTOR'S EXPENSE, REGARDLESS OF THEIR RESULTS, WHICH MAY BE SUCH THAT, IN THE CONSULTANT'S OPINION, THE WORK MAY BE ACCEPTABLE.
- ALL TESTING SHALL BE CONDUCTED IN ACCORDANCE WITH THE REQUIREMENTS OF THE BUILDING CODE, EXCEPT WHERE THIS WOULD, IN THE CONSULTANT'S OPINION, CAUSE UNDUE DELAY OR GIVE RESULTS NOT REPRESENTATIVE OF THE REJECTED MATERIAL IN PLACE. IN THIS CASE, THE TESTS SHALL BE CONDUCTED IN ACCORDANCE WITH THE STANDARDS GIVEN BY THE CONSULTANT.
- MATERIALS OR WORK, WHICH FAIL TO MEET SPECIFIED REQUIREMENTS, MAY BE REJECTED BY THE CONSULTANT WHENEVER FOUND AT ANY TIME PRIOR TO FINAL ACCEPTANCE OF THE WORK REGARDLESS OF PREVIOUS INSPECTION. IF REJECTED, DEFECTIVE MATERIALS OR WORKMANSHIP SHALL BE PROMPTLY REMOVED AND REPLACED OR REPAIRED TO THE SATISFACTION OF THE CONSULTANT, AT NO EXPENSE TO THE OWNER.

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3	ISSUED FOR PROGRESS	2024/06/04
2	ISSUED FOR PROGRESS	2024/05/03
1	MOH SUBMISSION	2023/10/18
NO	DESCRIPTION	DATE
SHEET REVISION		

PROJECT:  
THP CANCER CARE EQUIPMENT  
2200 Eglinton Ave W,  
Mississauga, ON L5M 2N1

TITLE:  
GENERAL NOTES

PROJECT NO:  
EN023-01052  
CHECKED:  
BW

DRAWING NO:

S001



ABBREVIATIONS			
A, ABV	ABOVE	JT	JOINT
A, ROD	ANCHOR ROD		
AEC	ARCH EXPOSED CONCRETE	Ed	DEVELOPMENT LENGTH OF REINFORCEMENT
AESS	ARCH EXPOSED STRUCTURAL STEEL		
AHU	AIR HANDLING UNIT	LG	LONG
ALT	ALTERNATE	LL	LOWER LAYER
ARCH	ARCHITECTURAL	LLH	LONG LEG HORIZONTAL
ASL	ACCUMULATED SNOW LOAD	LLV	LONG LEG VERTICAL
		LO	LOWER OF TWO BEAMS
		LP	LOW POINT
B, BOT	BOTTOM	LSH	LONG SIDE HORIZONTAL
BL	BOTTOM LOWER LAYER	LSV	LONG SIDE VERTICAL
BM	BEAM		
BML	BOTTOM MIDDLE LAYER		
BOF	BOTTOM (FOUNDING ELEVATION) OF FOOTING	MAX	MAXIMUM
		MC	MOMENT CONNECTION
BPL	BASE (BEARING) PLATE	MECH	MECHANICAL
BUL	BOTTOM UPPER LAYER	MEZZ	MEZZANINE
		MIN	MINIMUM
C	EPOXY COATED	MI	MOVEMENT JOINT
C/C	CENTRE TO CENTRE	ML	MIDDLE LAYER
C/W	CONNECT WITH	MOM	MOMENT
CA, CB	COLUMN ABOVE, BELOW		
CANT	CANTILEVER	N-S	NORTH-SOUTH
CJ	CONSTRUCTION JOINT	NIC	NOT IN CONTRACT
CLR	CLEAR	NF	NEAR FACE
CLS	COMPRESSION LAP SPLICE	No	NUMBER
COL	COLUMN	NTS	NOT TO SCALE
CONC	CONCRETE		
CONT	CONTINUOUS	OF	OUTSIDE FACE
COS	CONFIRM ON SITE	OPEN	OPENING
CP	CAST-IN PLATE	OWSJ	OPEN WEB STEEL JOIST
CSS	CHANGE IN SPANNING SYSTEM	PA, PB	POST ABOVE, BELOW
		PC	PRECAST
DB	DIVIDER BEAM	PL	PLATE
DET	DETAIL	PROJ	PROJECTION
DIA, Ø	DIAMETER		
DIAG	DIAGONAL		
DIM	DIMENSION	R	RADIUS
DWG(S)	DRAWING(S)	R/W	REINFORCE WITH
DWL(S)	DOWEL(S)	RD	ROOF DRAIN
		REF	REFERENCE
E-W	EAST-WEST	REINF	REINFORCE, REINFORCEMENT
EA	EACH	REQ'D	REQUIRED
EE	EACH END	REV	REVISION, REVISED
EF	EACH FACE	RTU	ROOFTOP UNIT
EJ, EXP JT	EXPANSION JOINT		
EL	ELEVATION	SA	SHELF ANGLE
ELECT	ELECTRICAL	SDF	STEP DOWN FOOTING
ELEV	ELEVATOR	SL	SLAB
EQ	EQUAL	SOG	SLAB-ON-GRADE
ES	EACH SIDE	SPEC'S	SPECIFICATIONS
EW	EACH WAY	SQ	SQUARE
EX, EXIST	EXISTING	STD	STANDARD
EXT	EXTERIOR	STRUCT	STRUCTURAL
		SW	SELF WEIGHT
f'c	CONCRETE COMPRESSIVE STRENGTH	1, THK	THICKNESS
FDN	FOUNDATION	T	TOP
Ff	FAR FACE	TD	TYPICAL DETAIL
FIN	FINISHED	TEMP	TEMPERATURE
FL	FLOOR	TJ	TIE JOIST
FMC	FULL MOMENT CONNECTION	TLL	TOP LOWER LAYER
FTG	FOOTING	TLS	TENSION LAP SPLICE
fy	YIELD STRENGTH FOR STEEL REINFORCEMENT	TML	TOP MIDDLE LAYER
		TOW	TOP OF WALL
Fy	YIELD STRENGTH FOR STRUCTURAL STEEL	TUL	TOP UPPER LAYER
		TYP	TYPICAL
GA	GAUGE	U/N, UNO	UNLESS NOTED OTHERWISE
GALV	GALVANIZED	U/S	UNDERSIDE
		UL	UPPER LAYER
H, HORIZ	HORIZONTAL	V, VERT	VERTICAL
HA, HB	HANGER ABOVE, BELOW	VBR	VERTICAL BRACING
HEF	HORIZONTAL EACH FACE	VEF	VERTICAL EACH FACE
HH	HOOKED EACH END	VSC	VERTICALLY SLOTTED CONNECTION
HI	HIGHER OF TWO BEAMS		
HP	HIGH POINT		
HSC	HORIZONTALLY SLOTTED CONNECTION		
		W/	WITH
IF	INSIDE FACE	WP	WORK POINT
INT	INTERIOR	WPA, WPB	WIND POST ABOVE, BELOW
IR	INTEGRITY REINFORCEMENT	WPL	WALL PLATE
		WWF	WELDED WIRE FABRIC

SYMBOLS			
@	AT	200 SLAB	200 SOG
CL	CENTRE LINE	BH-123	MIN-123
JL	DOUBLE ANGLE		
MC	MOMENT CONNECTION		
L	SINGLE ANGLE	CSS CSS	
VZ	VERTICAL CONTROL JOINT IN MASONRY/CONCRETE WALLS		
±XX	BEAM ELEVATION FROM PLAN DATUM		
SDF	STEPPED-DOWN FOOTING		
WP	WORK POINT		

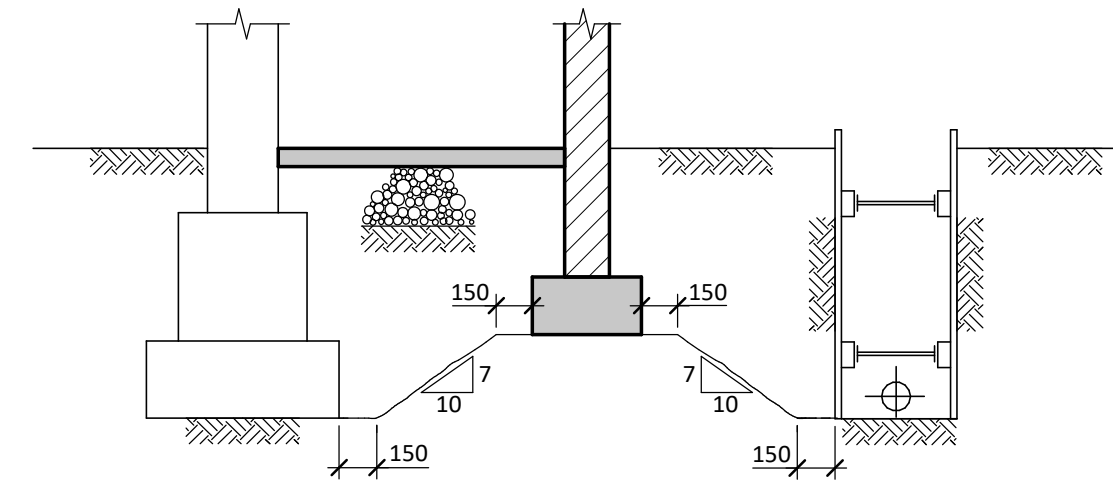
G1	ABBREVIATIONS AND SYMBOLS
----	---------------------------

FORCES	
BR	FACTORED BEARING FORCE, kN
DL	DEAD LOAD
H	FACTORED END SHEAR FORCE ALONG X-X AXIS (Vx), kN
LL	LIVE LOAD
M, Mx	FACTORED BENDING MOMENT ABOUT X-X AXIS, kN-m
My	FACTORED BENDING MOMENT ABOUT Y-Y AXIS, kN-m
P+	FACTORED TENSION AXIAL FORCE, kN
P-	FACTORED COMPRESSION AXIAL FORCE, kN
P±	FACTORED TENSION / COMPRESSION (REVERSIBLE) FORCE, kN
PTF	FACTORED PASS THROUGH FORCE, kN
TM	FACTORED TORSIONAL MOMENT, kN-m
V	FACTORED END SHEAR FORCE ALONG Y-Y AXIS (Vy), kN
WD	UNFACTORED UNIFORMLY DISTRIBUTED DEAD LOAD ON STEEL DECK, PRECAST OR OWSJ's
WL	UNFACTORED UNIFORMLY DISTRIBUTED LIVE LOAD ON STEEL DECK, PRECAST OR OWSJ's

UNITS	
kg	KILOGRAM
m	METRE
kPa	PASCAL
kN/m <sup>2</sup>	KILONEWTON PER SQUARE METRE (KILOPASCAL)
MPa	MEGAPASCAL
N	NEWTON
kN	KILONEWTON
kN-m	KILONEWTON METRE
kN/m	KILONEWTON PER METRE

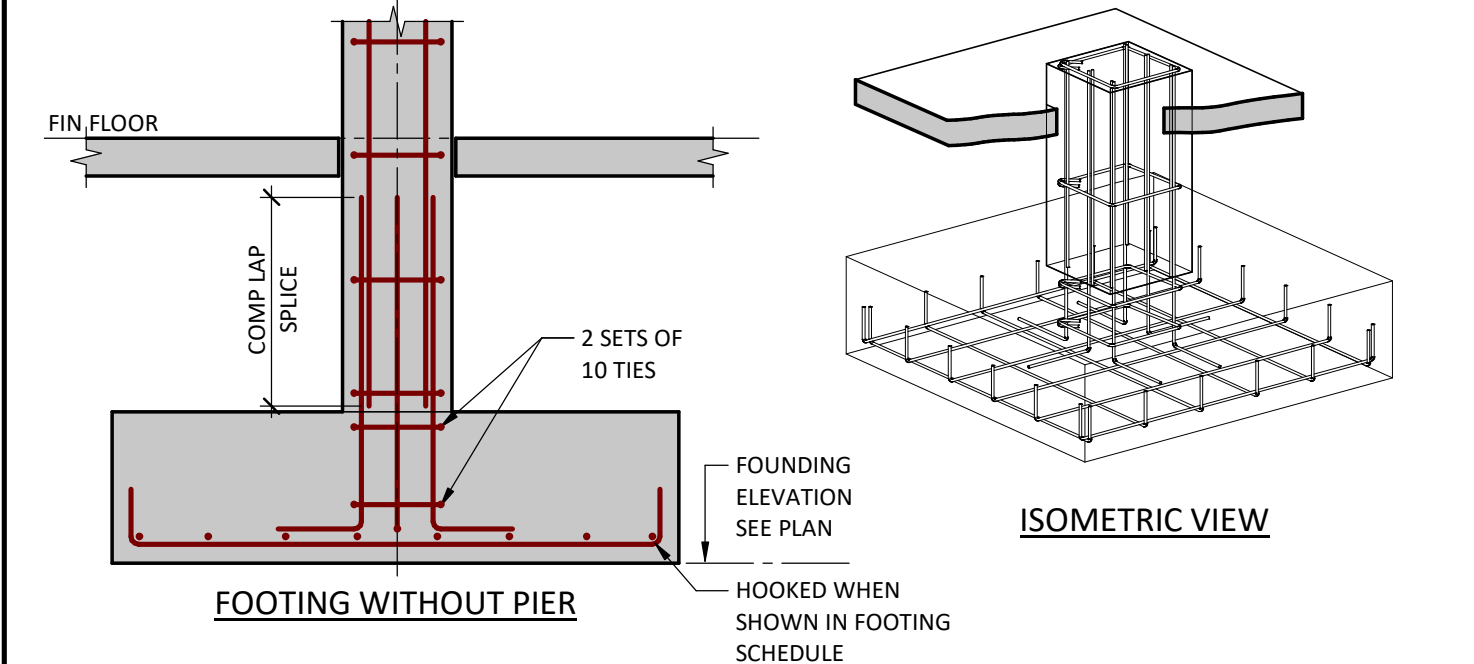
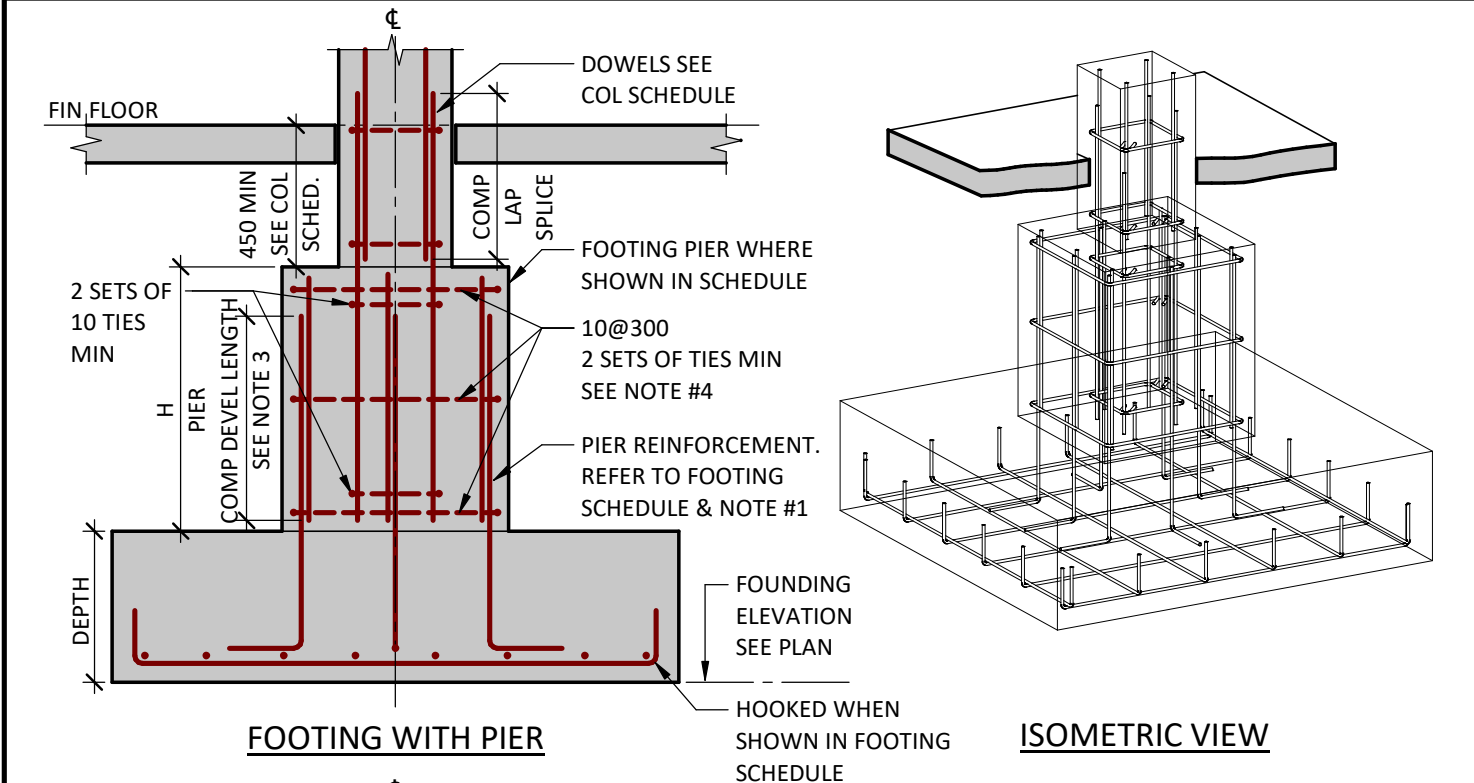
MARK		INDICATIVE OF
NEW	EXISTING	
		CONCRETE / MASONRY / WOOD STRUCTURE BELOW
		CONCRETE STRUCTURE ABOVE
		CONCRETE UPTURNED BEAMS AND CURBS
		CONCRETE IN SECTION
		MASONRY WALL ABOVE AND IN SECTION
		WOOD SHEAR WALL ABOVE
		WOOD LOAD BEARING WALL ABOVE

G2	STRUCTURE LEGEND
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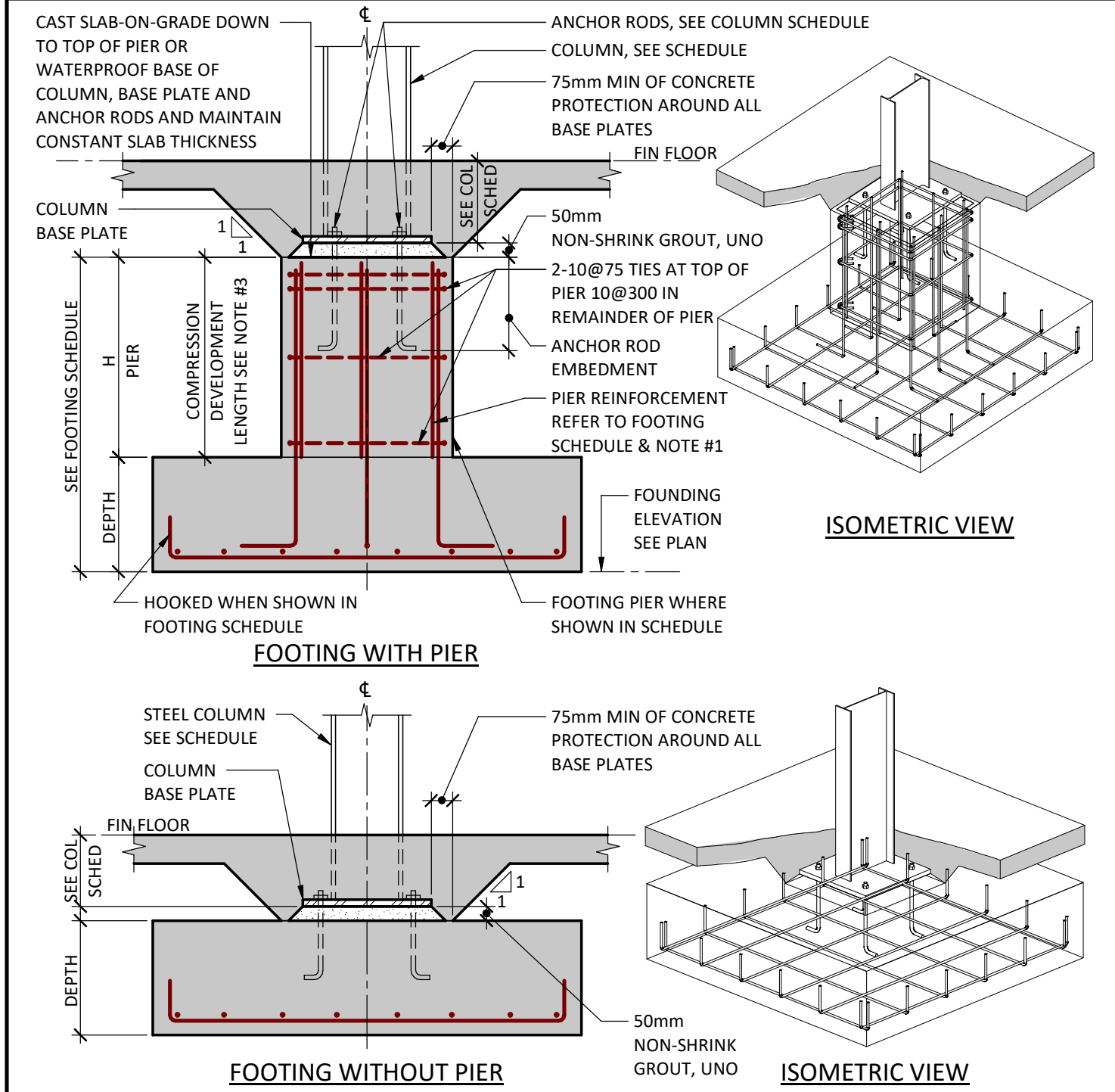
- NOTES:
1. MAINTAIN MINIMUM SLOPE TO ADJACENT FOOTINGS AND EXCAVATIONS UNLESS NOTED OTHERWISE IN THE GEOTECHNICAL REPORT.
  2. LOWER FOOTINGS AS NECESSARY TO ACHIEVE SLOPE NOTED.
  3. ENSURE SPECIFIED SLOPE IS NOT COMPROMISED BY DISTURBED SOIL BETWEEN THE EXCAVATIONS.
  4. WHERE FOOTINGS ARE STEPPED DOWN, A MAXIMUM LEVEL DIFFERENCE OF 600mm SHOULD BE MAINTAINED.

CF3	ADJACENT FOOTINGS AND EXCAVATIONS
-----	-----------------------------------



- NOTES:
1. PROVIDE MINIMUM VERTICAL STEEL OF 0.5% OF PIER AREA IN PIER WITH TIES (8-20 MIN) UNLESS NOTED OTHERWISE IN SCHEDULE.
  2. PROVIDE DEVELOPMENT AND SPLICE LENGTHS AS SHOWN, UNLESS NOTED OTHERWISE.
  3. EXTEND DOWELS INTO FOOTING BELOW WHEN HEIGHT OF PIER IS LESS THAN THE COMPRESSION DEVELOPMENT LENGTH OF THE DOWELS BEING DEVELOPED AND WHERE NO PIER EXISTS.
  4. ARRANGE VERTICAL BARS AND DETAIL SETS OF TIES AROUND DOWELS IN ACCORDANCE WITH TYPICAL DETAIL CC1.

CF5	FOOTING UNDER CONCRETE COLUMN
-----	-------------------------------



- NOTES:
1. PROVIDE MINIMUM VERTICAL STEEL OF 0.5% OF PIER AREA IN PIER WITH TIES (8-20 MIN) UNLESS NOTED OTHERWISE IN SCHEDULE.
  2. PROVIDE DEVELOPMENT AND SPLICE LENGTHS AS SHOWN, UNLESS NOTED OTHERWISE.
  3. EXTEND DOWELS INTO FOOTING BELOW WHEN HEIGHT OF PIER IS LESS THAN THE COMPRESSION DEVELOPMENT LENGTH OF THE DOWELS BEING DEVELOPED.
  4. ARRANGE VERTICAL BARS AND DETAIL SETS OF TIES AROUND DOWELS IN ACCORDANCE WITH TYPICAL DETAIL CC1.

CF6	FOOTING UNDER STEEL COLUMN
-----	----------------------------

TYPICAL DETAIL LIST		
DETAIL NO.	DETAIL TITLE	DRAWING NO.
CONCRETE FOOTINGS		
CF3_en	ADJACENT FOOTINGS AND EXCAVATIONS	S010
CF4	BACKFILL AROUND SERVICES BENEATH STRIP FOOTINGS	S011
CF5	FOOTING UNDER CONCRETE COLUMN	S010
CF6	FOOTING UNDER STEEL COLUMN	S010
CF8	CAISSON SUPPORTING CONCRETE/STEEL COLUMN	S011
CF9	EXCAVATION SUPPORT SYSTEMS & FOUNDATION WALLS	S011
CF11	UNDERPINNING DETAILS	S012
CONCRETE SLABS		
CS11	REINFORCEMENT AT STEPS IN SLAB-ON-GRADE	S012
CS20	DETAILS FOR HOUSEKEEPING PADS	S012
CONCRETE WALLS		
CW1	PITS AND TRENCHES	S011
CW2	REINFORCEMENT DETAILS IN CONCRETE WALLS	S011
GENERAL		
G1_en	ABBREVIATIONS AND SYMBOLS	S010
G2_en	STRUCTURE LEGEND	S010
MASONRY		
M1	LINTELS FOR NON-LOAD BEARING MASONRY WALLS	S012

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NO	DESCRIPTION	DATE

PROJECT:  
THP CANCER CARE EQUIPMENT  
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Mississauga, ON L5M 2N1

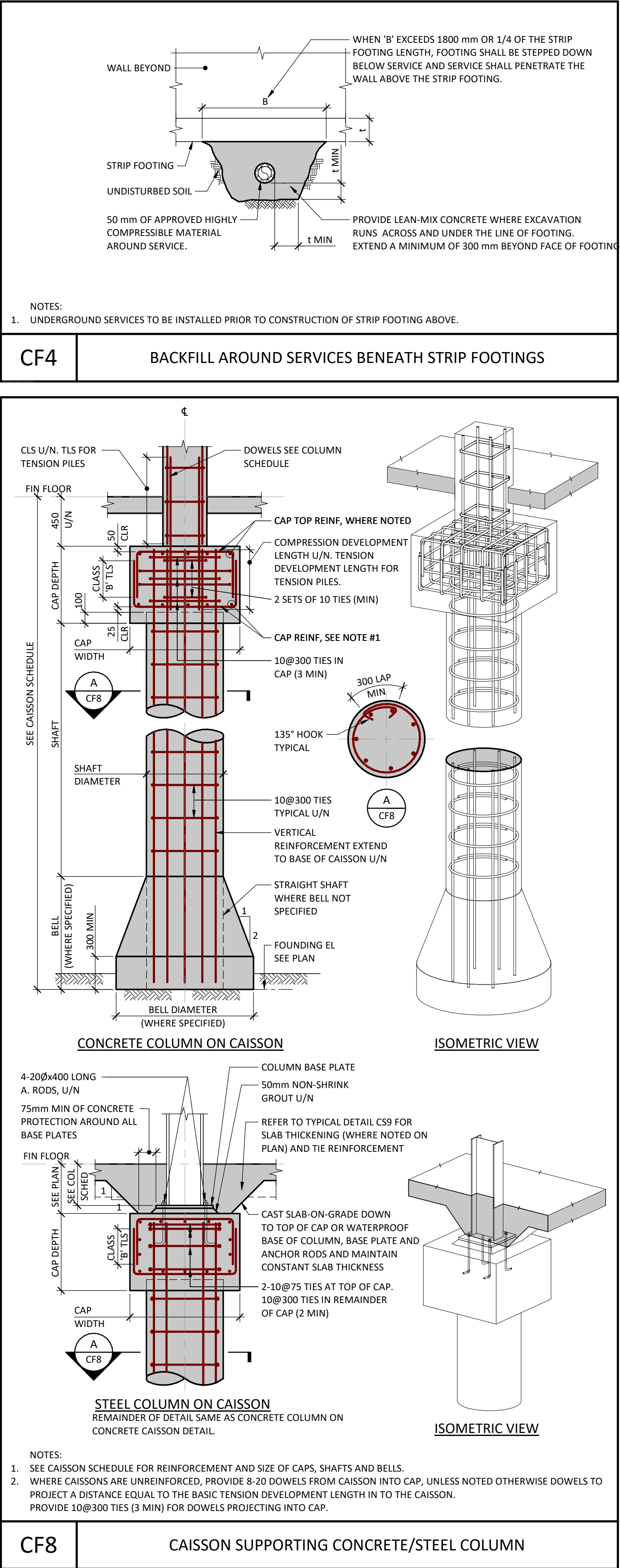
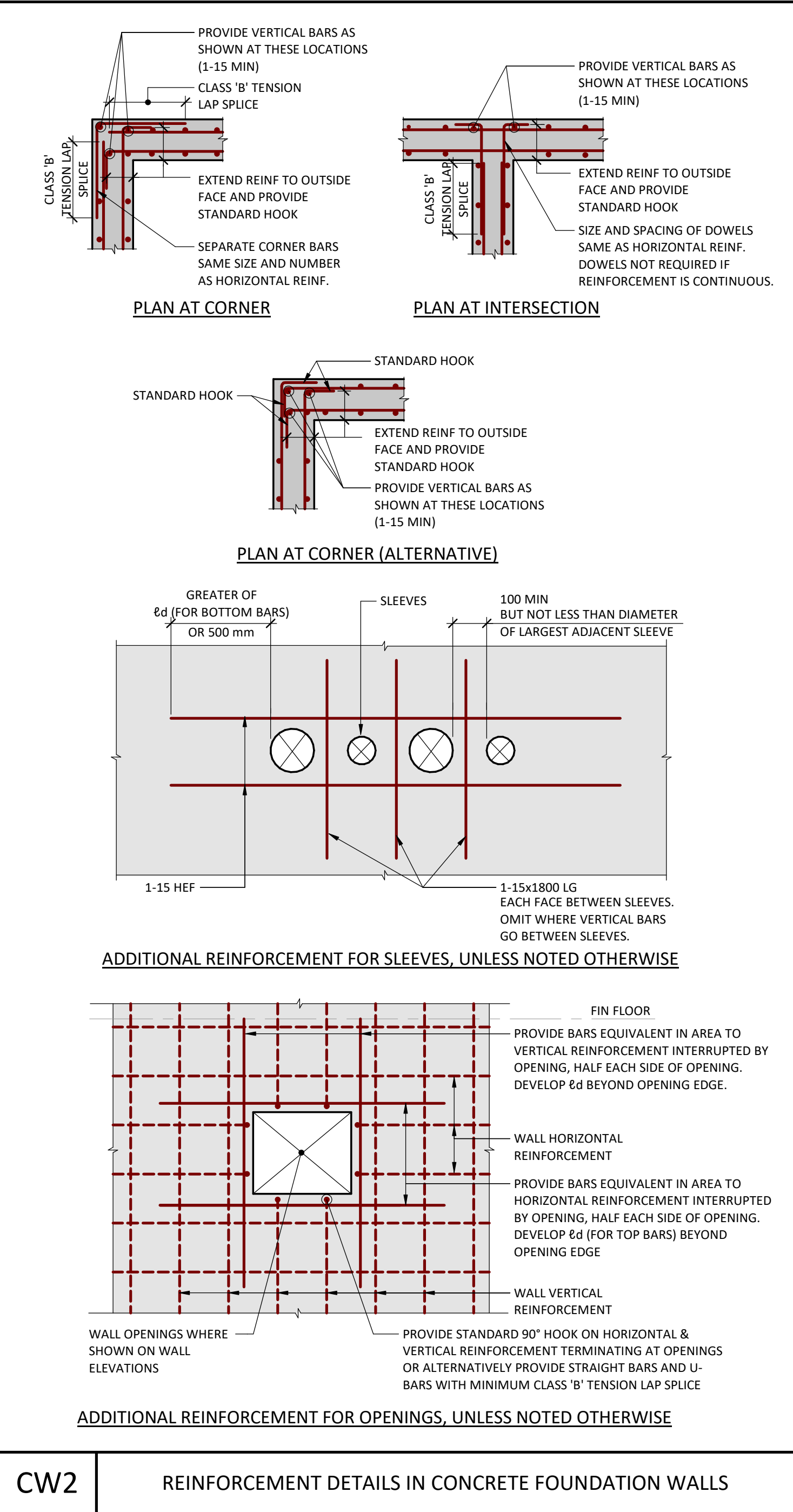
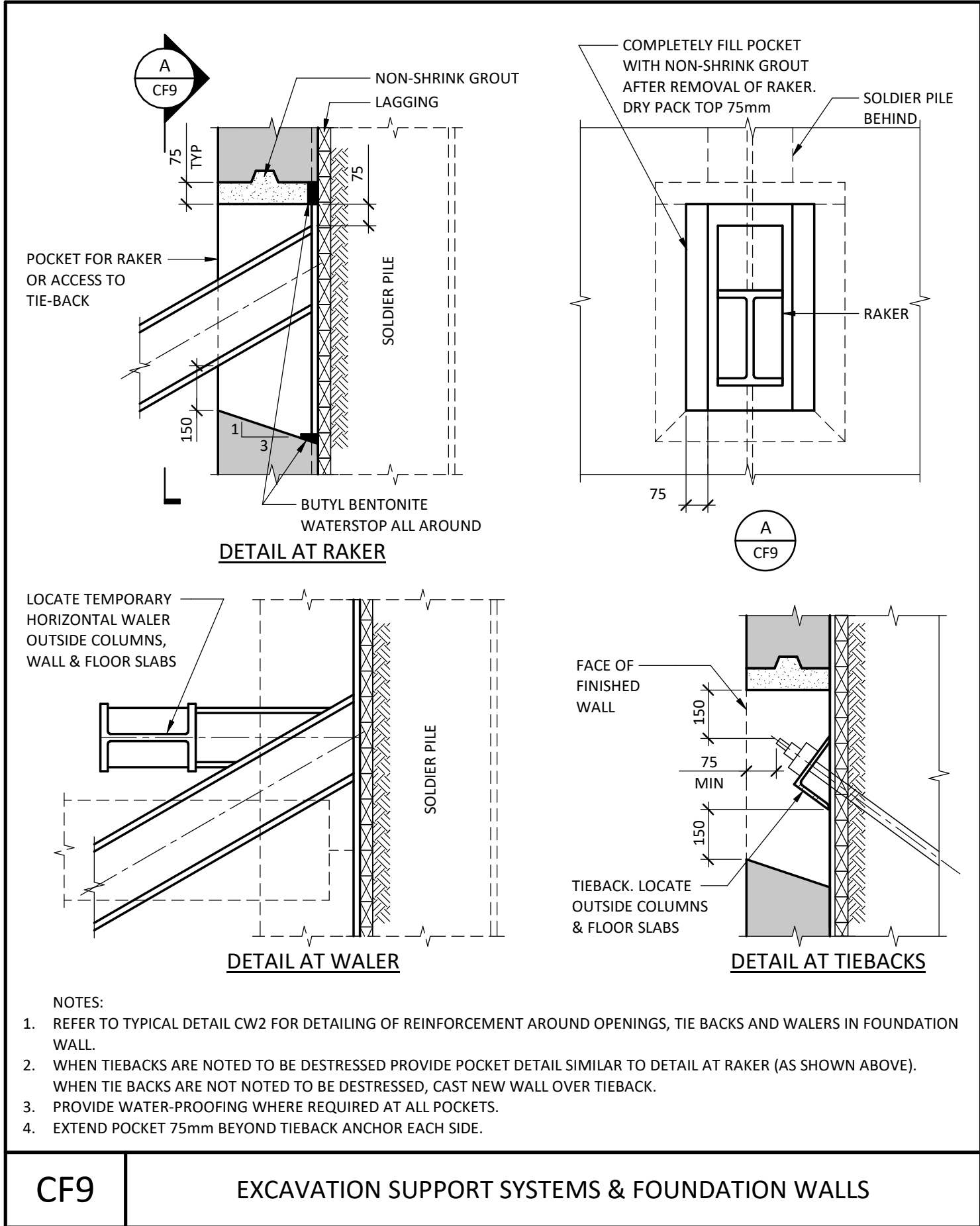
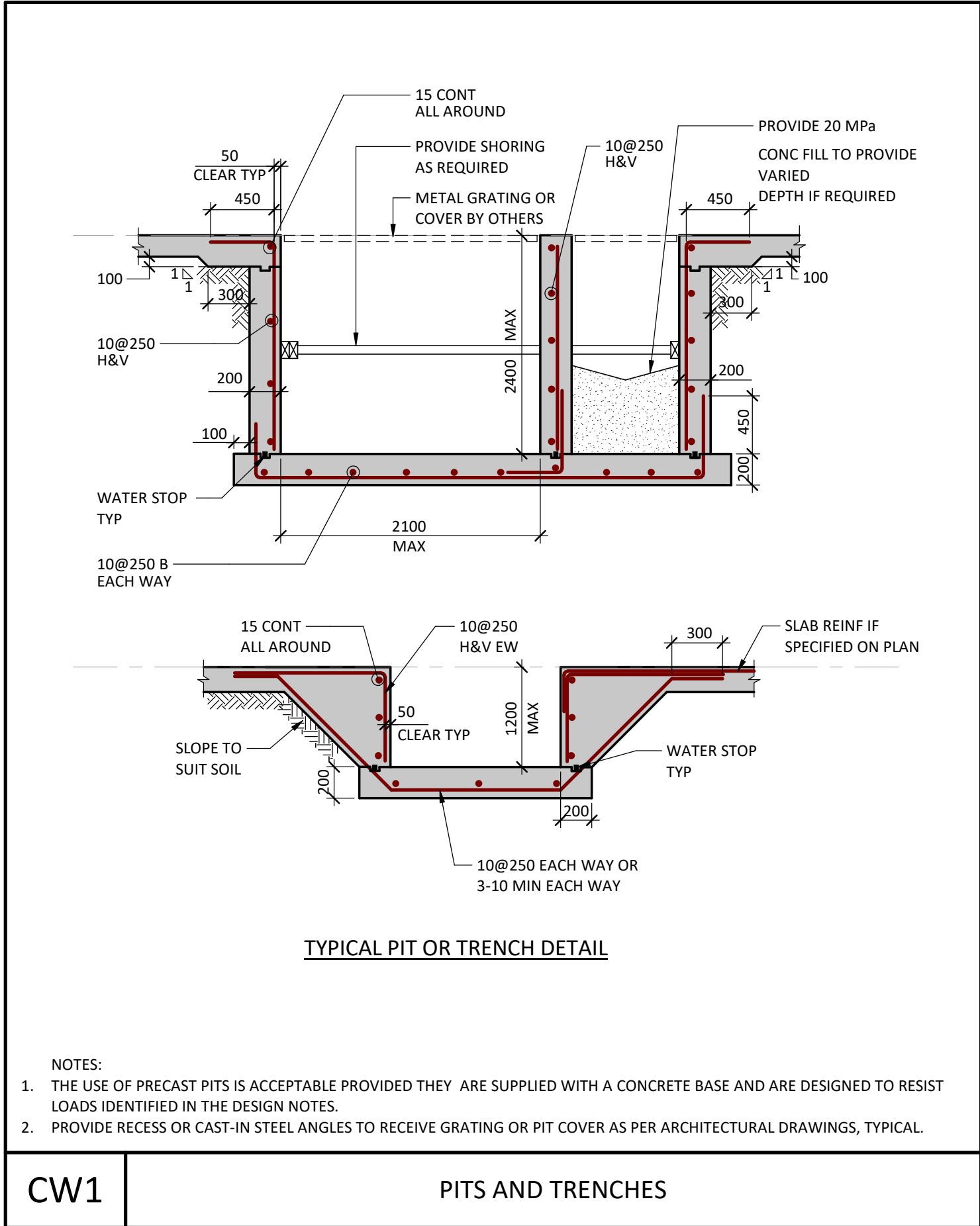
TITLE:  
**TYPICAL DETAILS**

PROJECT NO:  
EN023-01052

CHECKED:  
BW

DRAWING NO:  
**S010**





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Y. K. WONG  
100161713  
2025-12-16  
PROVINCE OF ONTARIO

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PROJECT:

TYP CANCER CARE EQUIPMENT  
2200 Eglinton Ave W,  
Mississauga, ON L5M 2N1

TITLE:

TYPICAL DETAILS

PROJECT NO:

EN023-01052

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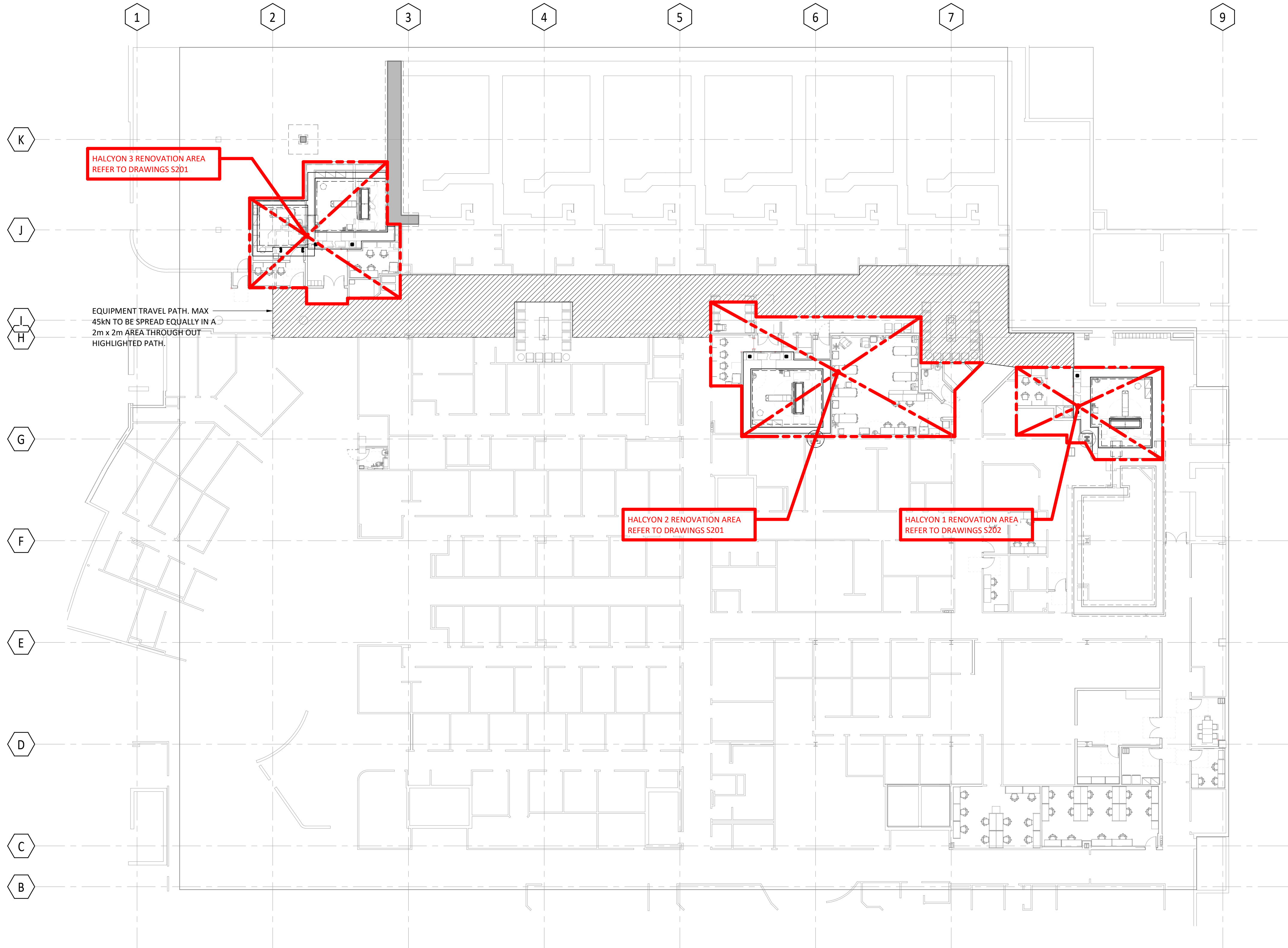
BW

DRAWING NO:

S011







OVERALL FRAMING PLAN - LEVEL 1  
1 : 200

- 1. FINISHED FLOOR IS AT ELEVATION 161.0 EXCEPT AS CROSSED AND NOTED. ELEVATIONS FOR AREAS CROSSED AND NOTED ARE TO BE READ FROM FINISHED FLOOR ELEVATION.
- 2. EXISTING TOP OF INTERIOR CAISSONS AT ELEVATION 160.0 UNLESS NOTED OTHERWISE AS NOTED IN THE EXISTING STRUCTURAL DRAWINGS.
- 3. EXISTING TOP OF PERIMETER CAISSONS AT ELEVATION 159.65 UNLESS NOTED IN THE EXISTING STRUCTURAL DRAWINGS.
- 4. EXISTING TOP OF COLUMN FOOTINGS AT ELEVATION 160.0 AND WALL FOOTINGS AT ELEVATION 160.8 UNLESS NOTED OTHERWISE.
- 5. ALLOWANCE TO BE PROVIDED TO ENSURE SOIL CONDITIONS BELOW EXISTING FOUNDATIONS ARE NOT DISTURBED DURING CONSTRUCTION.
- 6. TOP OF NEW FOOTING IS AT ELEVATION 160.5 EXCEPT AS CROSSED AND NOTED. ELEVATIONS FOR AREAS CROSSED AND NOTED ARE TO BE READ FROM FINISHED FLOOR ELEVATION.
- 7. PROVIDE PRE-CONDITION ASSESSMENT OF THE EXISTING SOG SURROUNDING THE WORK AREAS. ROUT AND GROUT ANY NEW SOG CRACKS THAT RESULT FROM CONSTRUCTION ACTIVITIES.

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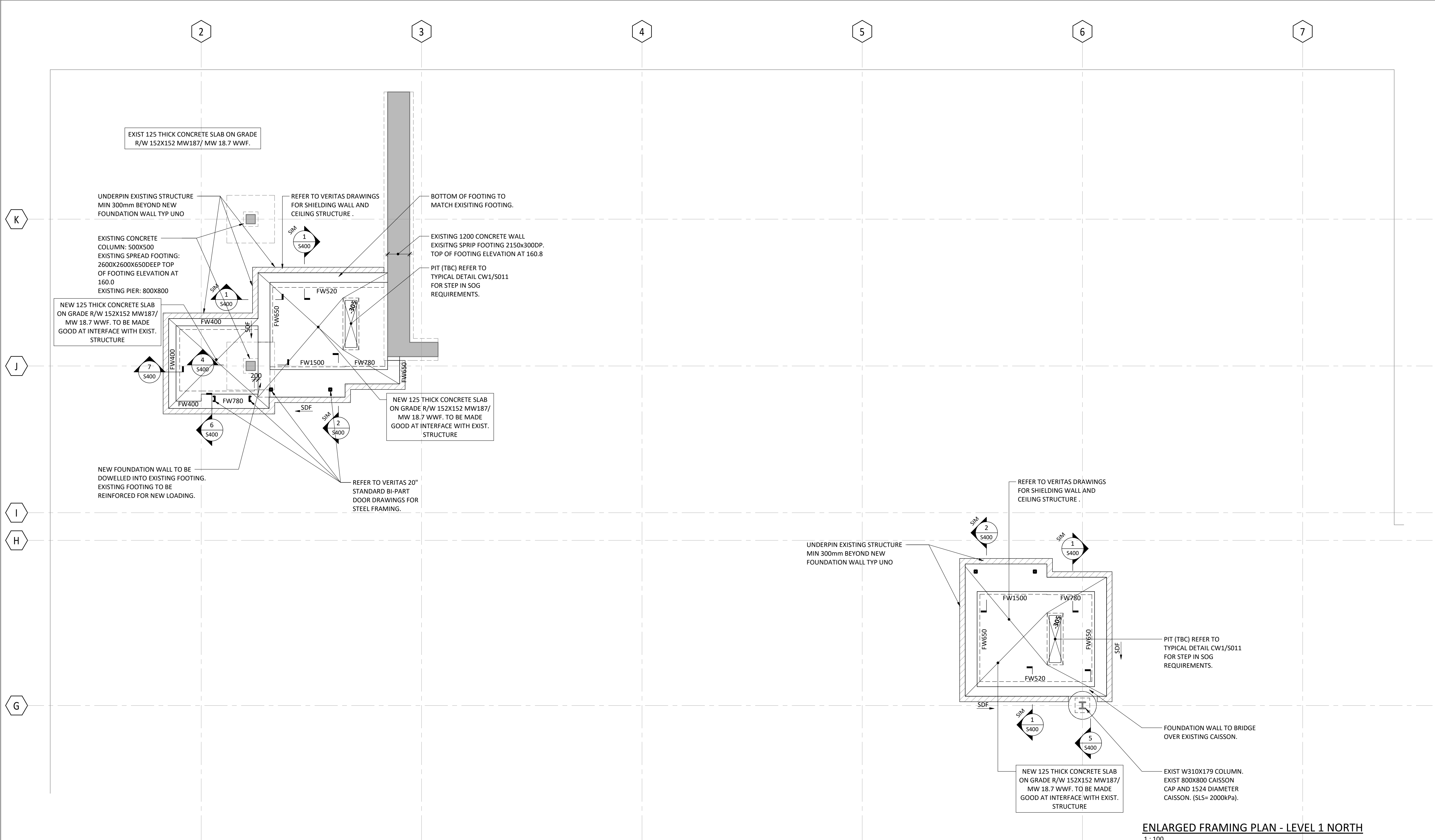
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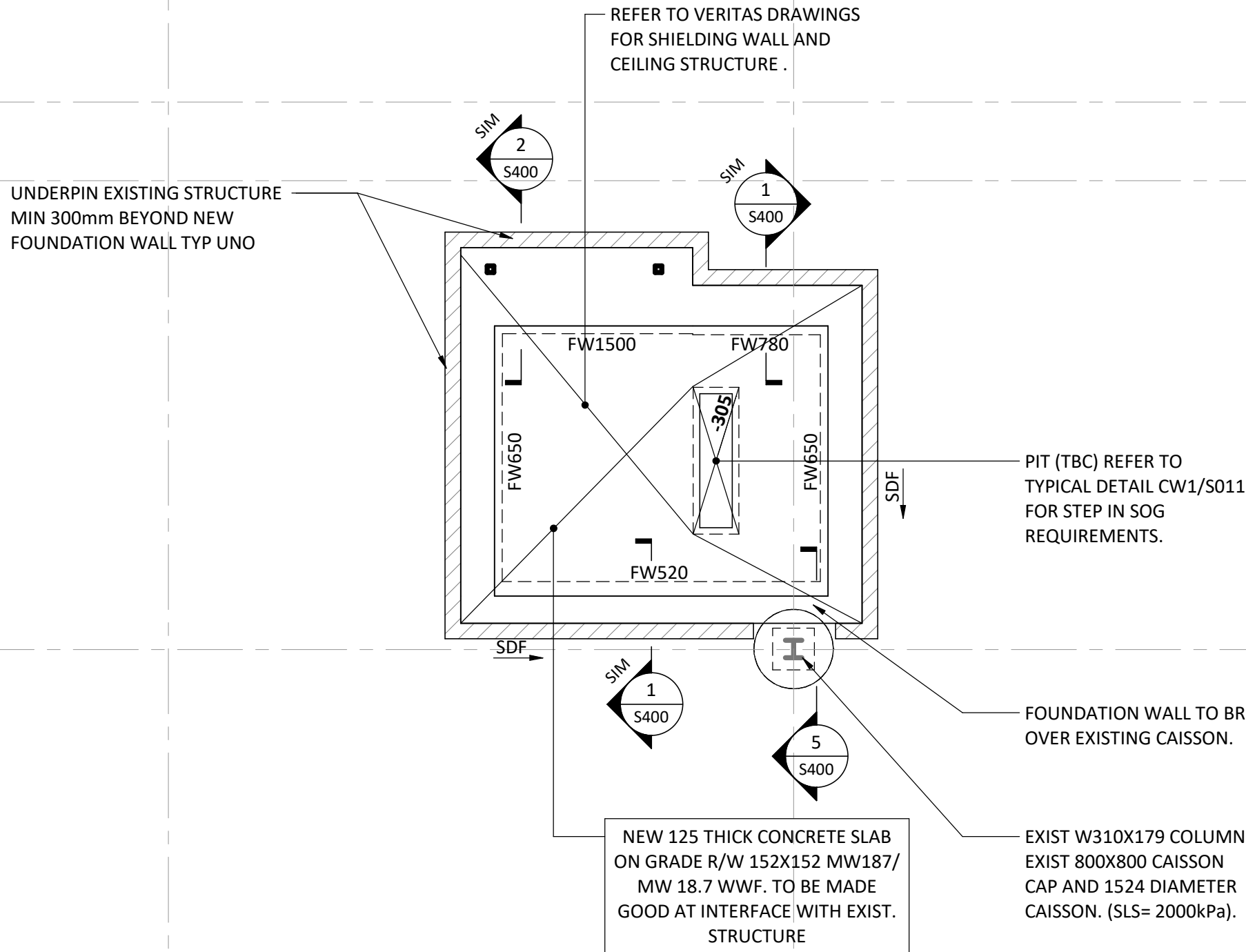
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1	MOH SUBMISSION	2023/10/18
NO	DESCRIPTION	DATE
SHEET REVISION		
PROJECT: THP CANCER CARE EQUIPMENT 2200 Eglinton Ave W, Mississauga, ON L5M 2N1		
TITLE: OVERALL FRAMING PLAN - LEVEL 1		
PROJECT NO: EN023-01052	DRAWING NO:	S200
CHECKED: BW		





TYPE	THICKNESS OF FOUNDATION WALL	FOUNDATION WALL FOOTING		WALL REINFORCEMENT
		WIDTH	DEPTH	
FW400	400	600	300	15@400 HEF & VEF
FW520	520	800	300	15@375 HEF & VEF
FW650	650	800	300	15@300 HEF & VEF
FW780	780	950	300	15@250 HEF & VEF
FW850	850	1000	300	15@225 HEF & VEF
FW910	910	1100	300	15@225 HEF & VEF
FW1500	1500	1650	300	20@200 HEF & VEF



ENLARGED FRAMING PLAN - LEVEL 1 NORTH  
1 : 100

- FINISHED FLOOR IS AT ELEVATION 161.0 EXCEPT AS CROSSED AND NOTED. ELEVATIONS FOR AREAS CROSSED AND NOTED ARE TO BE READ FROM FINISHED FLOOR ELEVATION.
- EXISTING TOP OF INTERIOR CAISSONS AT ELEVATION 160.0 UNLESS NOTED OTHERWISE AS NOTED IN THE EXISTING STRUCTURAL DRAWINGS.
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- ALLOWANCE TO BE PROVIDED TO ENSURE SOIL CONDITIONS BELOW EXISTING FOUNDATIONS ARE NOT DISTURBED DURING CONSTRUCTION.
- TOP OF NEW FOOTING IS AT ELEVATION 160.5 EXCEPT AS CROSSED AND NOTED. ELEVATIONS FOR AREAS CROSSED AND NOTED ARE TO BE READ FROM FINISHED FLOOR ELEVATION.
- PROVIDE PRE-CONDITION ASSESSMENT OF THE EXISTING SOG SURROUNDING THE WORK AREAS. ROUT AND GROUT ANY NEW SOG CRACKS THAT RESULT FROM CONSTRUCTION ACTIVITIES.

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THP CANCER CARE EQUIPMENT  
2200 Eglinton Ave W,  
Mississauga, ON L5M 2N1

TITLE:  
**ENLARGED FRAMING PLAN -  
LEVEL 1 NORTH**

PROJECT NO:

EN023-01052

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BW

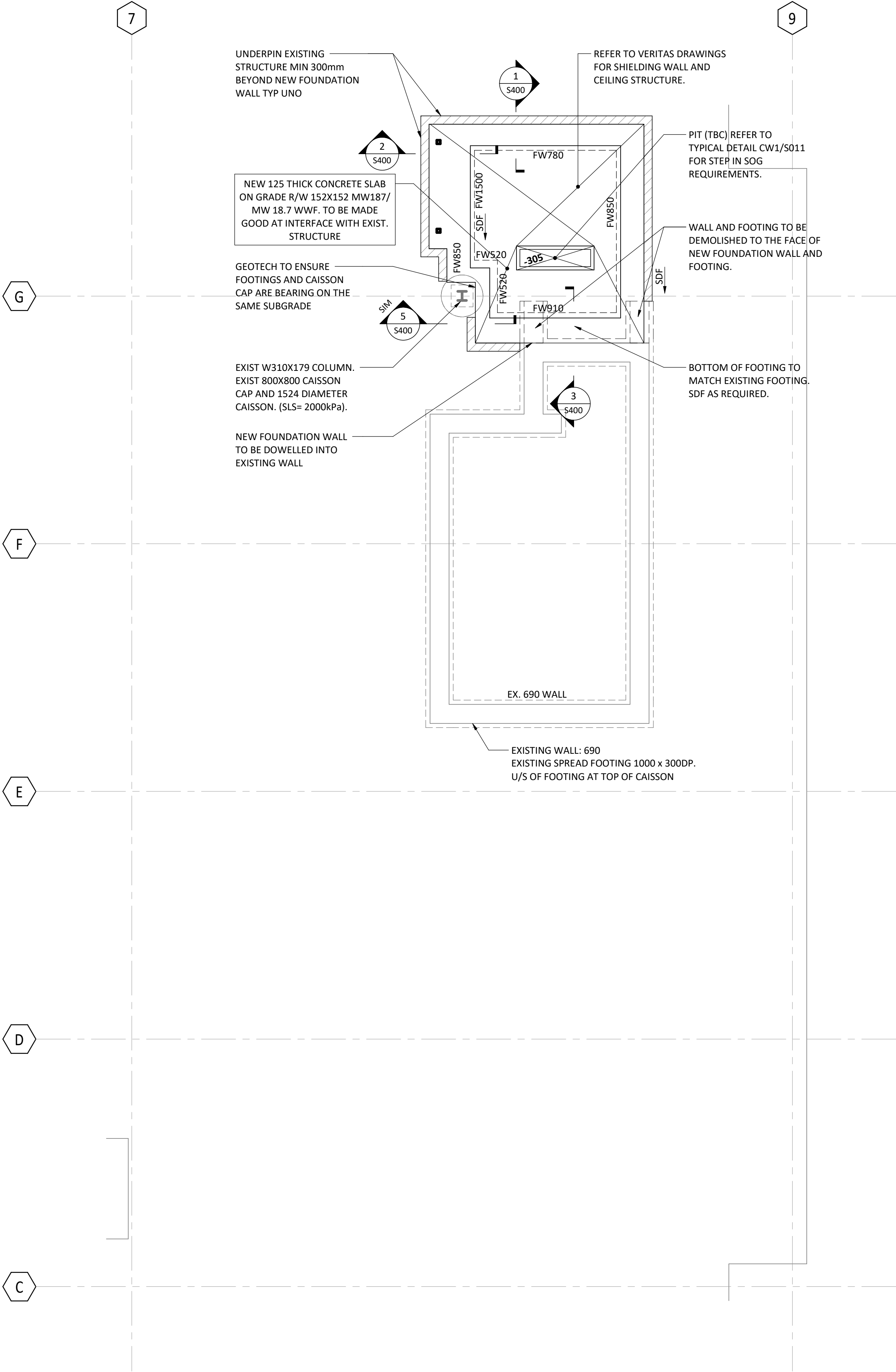
DRAWING NO:

**S201**

ENLARGED FRAMING PLAN - LEVEL 1 EAST

1 : 100

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6. TOP OF NEW FOOTING IS AT ELEVATION 160.5 EXCEPT AS CROSSED AND NOTED. ELEVATIONS FOR AREAS CROSSED AND NOTED ARE TO BE READ FROM FINISHED FLOOR ELEVATION.



TYPE	THICKNESS OF FOUNDATION WALL	FOUNDATION WALL FOOTING		WALL REINFORCEMENT
		WIDTH	DEPTH	
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FW520	520	800	300	15@375 HEF & VEF
FW650	650	800	300	15@300 HEF & VEF
FW780	780	950	300	15@250 HEF & VEF
FW850	850	1000	300	15@225 HEF & VEF
FW910	910	1100	300	15@225 HEF & VEF
FW1500	1500	1650	300	20@200 HEF & VEF

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1	MOH SUBMISSION	2023/10/18

PROJECT:  
THP CANCER CARE EQUIPMENT  
2200 Eglinton Ave W,  
Mississauga, ON L5M 2N1

TITLE:  
ENLARGED FRAMING PLAN -  
LEVEL 1 EAST

PROJECT NO:

EN023-01052

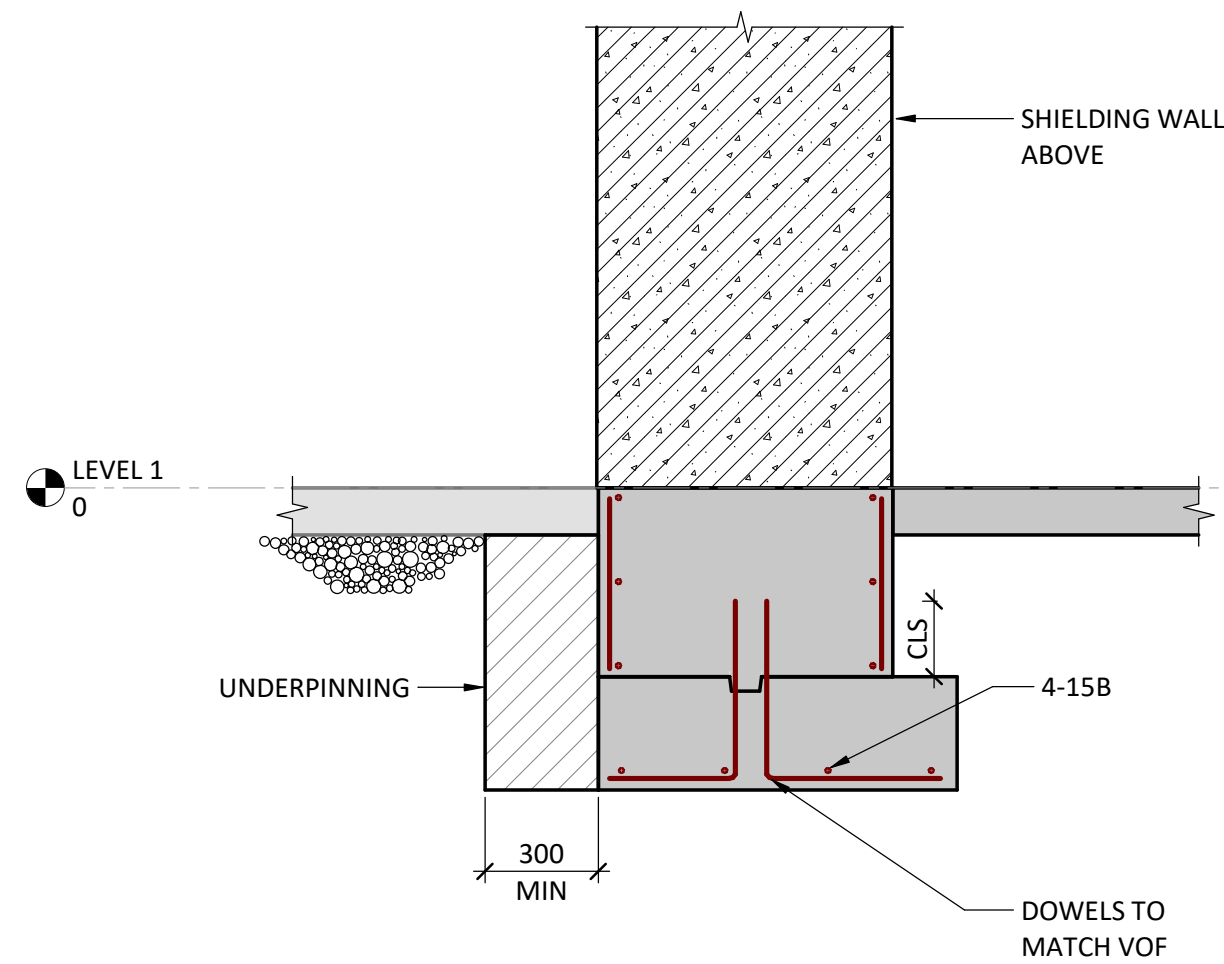
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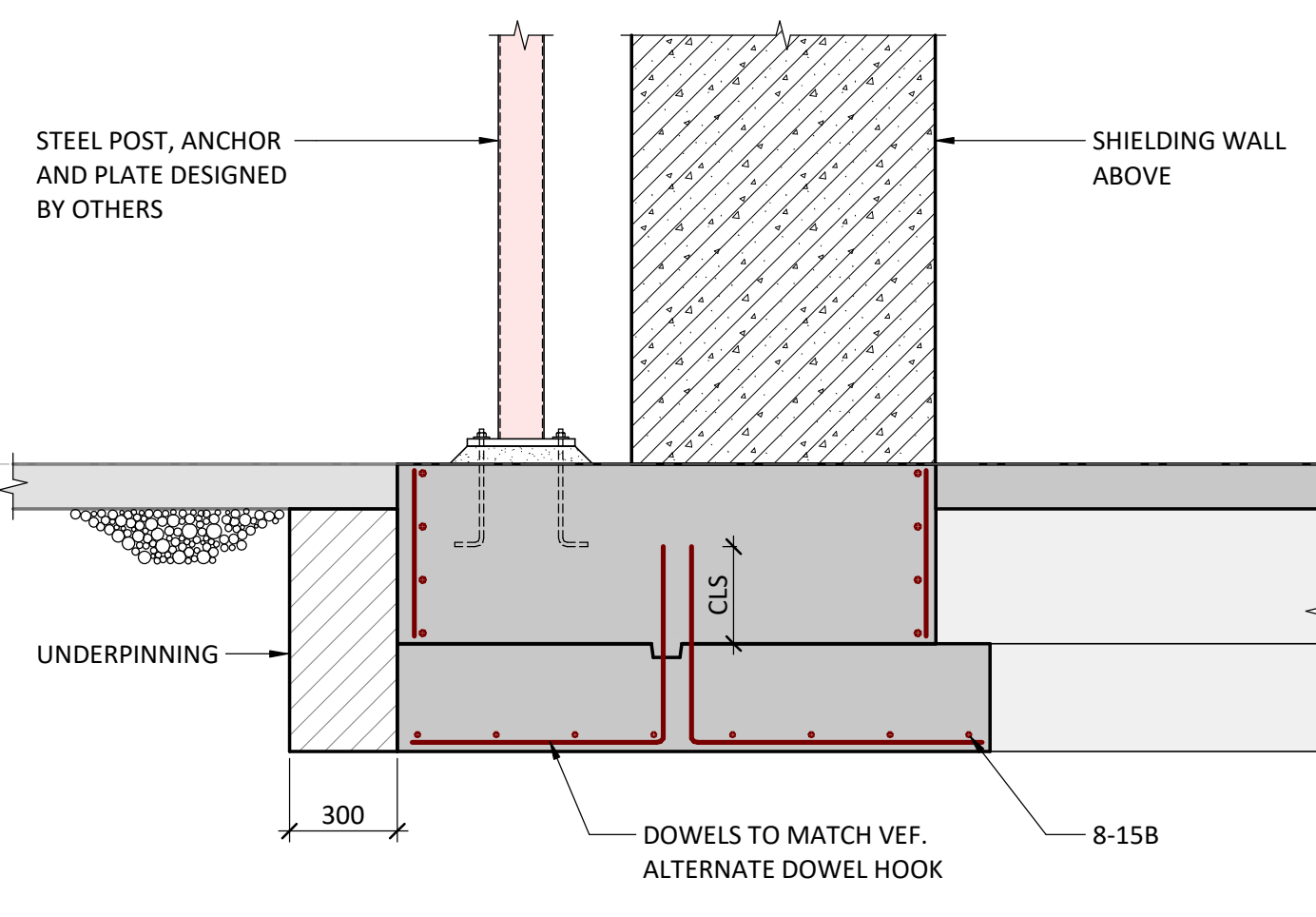
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S202

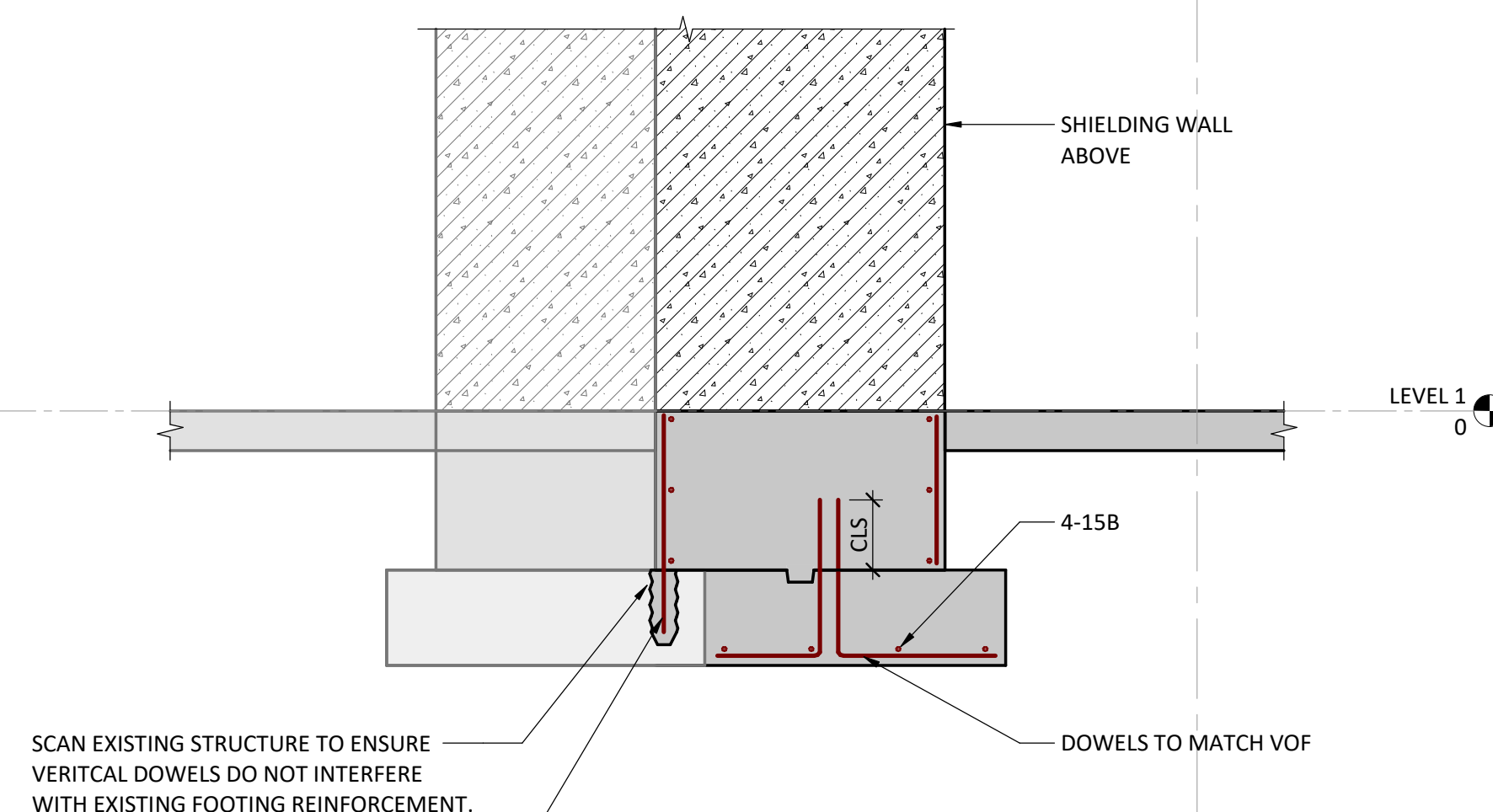




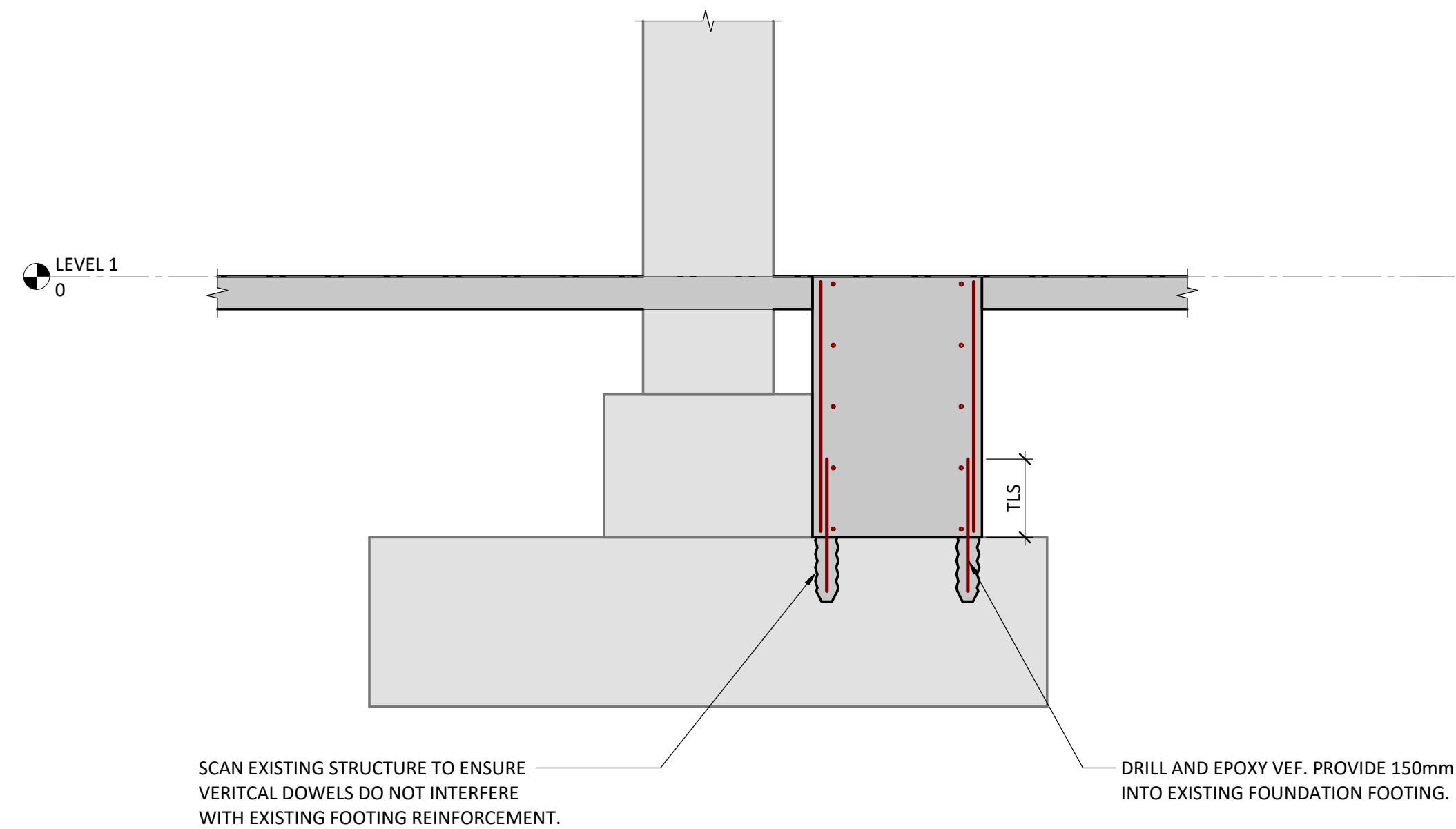
1  
S400  
1 : 20



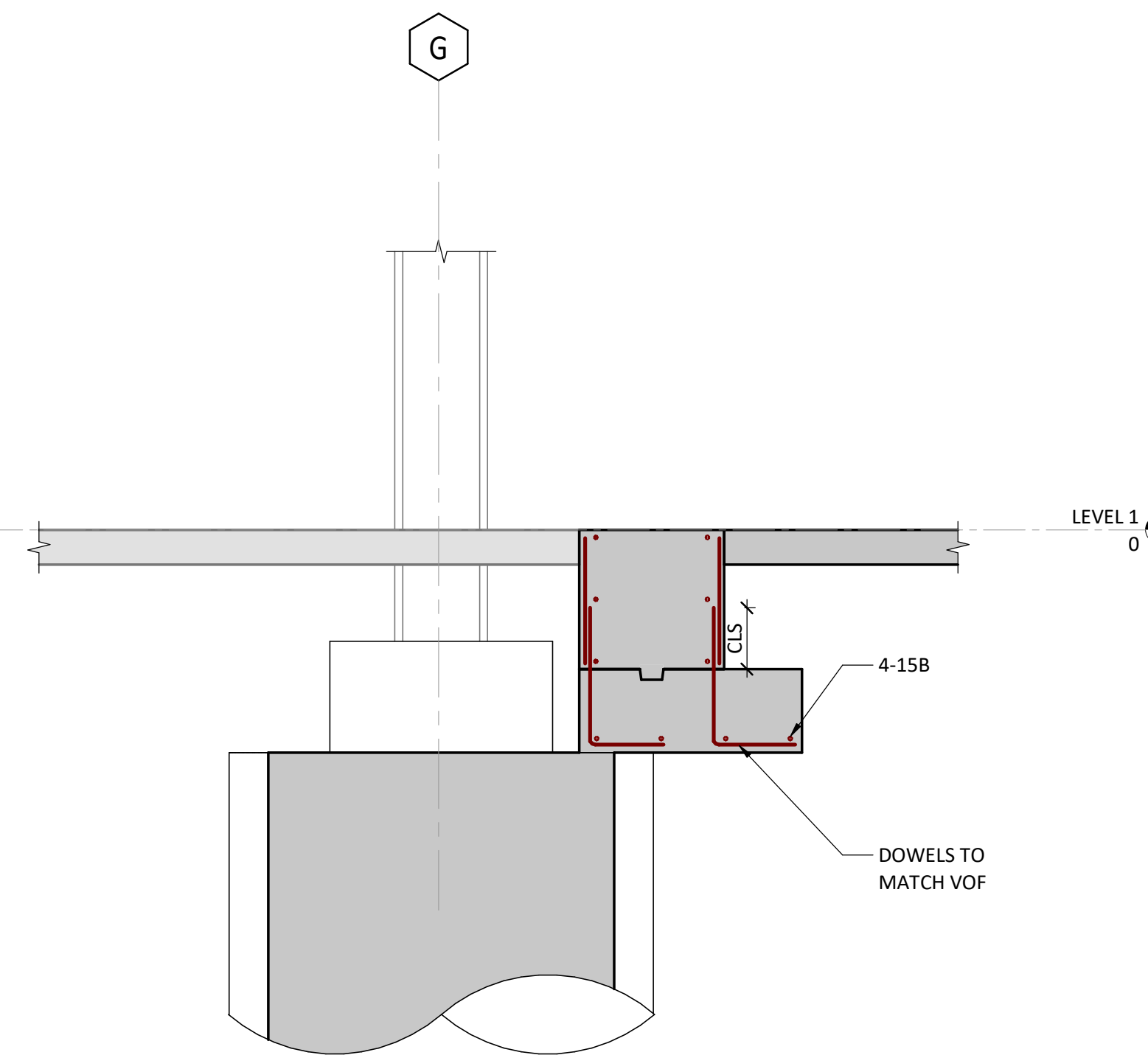
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S400  
1 : 20



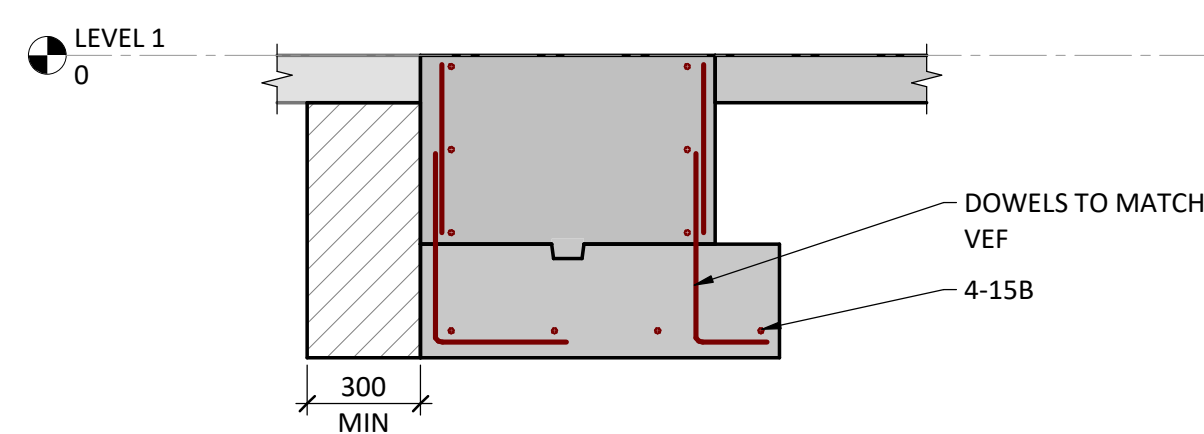
3  
S400  
1 : 20



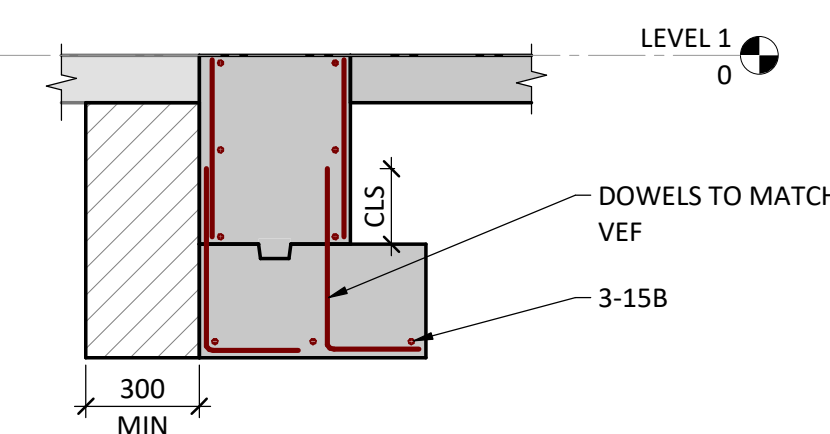
4  
S400  
1 : 20



5  
S400  
1 : 20



6  
S400  
1 : 20



7  
S400  
1 : 20

CLIENT:

CONSULTANT:

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NO	DESCRIPTION	DATE
9	ISSUED FOR TENDER	2025/12/16
8	ISSUED FOR PERMIT	2025/11/28
7	MOH 2.3 RE-SUBMISSION	2025/06/13
6	MOH 2.3 SUBMISSION	2024/10/11
5	ISSUED FOR 100% CONSTRUCTION DOCUMENTS	2024/09/13
4	MOH 2.3 COSTING SUBMISSION	2024/06/17
3	ISSUED FOR PROGRESS	2024/06/04
2	ISSUED FOR PROGRESS	2024/05/03
1	MOH SUBMISSION	2023/10/18

PROJECT:  
THP CANCER CARE EQUIPMENT  
2200 Eglinton Ave W,  
Mississauga, ON L5M 2N1

TITLE:  
**FOUNDATION SECTIONS & DETAILS**

PROJECT NO:  
EN023-01052  
CHECKED:  
BW

DRAWING NO:

**S400**